Schedule 'C' Municipal Class Environmental Assessment for Thirty Road (Regional Road 14) at Young Street in the Township of West Lincoln

APPENDIX

2 Detailed Transportation Assessment

If technical reports are required in an alternative format for accessibility needs, please contact:

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Thirty Road (Regional Road 14) & Young Street Class EA

Detailed Transportation Assessment

Final

Prepared for: Niagara Region

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RVA 226468

May 30, 2023

Hamed Esmaeeli, P.Eng., PhD Niagara Region 1815 Sir Isaac Brock Way, Thorold, ON L2V 4Y6 Hamed.Esmaeeli@niagararegion.ca

Attention: Hamed Esmaeeli, P.Eng., PhD

Dear Hamed:

Re: Thirty Road (Regional Road 14) & Young Street Class EA

Detailed Transportation Assessment

Please find attached our Detailed Transportation Assessment as part of the Thirty Road (Regional Road 14) & Young Street intersection Class Environmental Assessment (EA). If you have any questions about the study, please contact the undersigned at AHussain@rvanderson.com or (289) 348-1234 ext. 4516.

Yours very truly,

R.V. ANDERSON ASSOCIATES LIMITED

Michael Kong

Transportation Planner

Altaf Hussain, P.Eng.

Africa

Manager, Traffic and Transportation Planning



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1.0 Introduction

R.V. Anderson Associates (RVA) has been retained by the Niagara Region (Region) to complete a Schedule "C" Municipal Class Environmental Assessment (MCEA), Detailed Transportation Assessment, and Preliminary design for the potential re-alignment and intersection improvements at Thirty Road (Regional Road 14) and Young Street, in the Township of West Lincoln.

The intersection operated under two-way stop control (TWSC) with free-flow movement along Thirty Road up until November 2022, but has since been changed to all-way stop control (AWSC) as an interim measure. This temporary remedial measure was put in place as a response to significant resident concern regarding traffic safety, with key issues being insufficient sightlines due the reverse curve on Thirty Road, as well as skewed intersection alignment at Clayson Road and Thirty Road, which is a three-legged intersection just north of the subject intersection. An aerial view of the subject intersections can be seen in **Figure 1.1** below.



Figure 1.1 – Thirty Road Horizontal Alignment

Further to addressing safety issues, the proposed intersection alternatives must consider heavy vehicle accommodation as these corridors, particularly Young Street, serve as important routes for farm vehicles and trucks.

The findings of this report can be used to identify feasible improvement alternatives for the study area, with specific consideration given to enhancing sightlines and safety. The feasibility of each solution will also be appraised based on their ability to maintain the overall functionality of the arterial roadways.

2.0 Existing (2022) Conditions

2.1 Study Area Road Network

The study area, inclusive of Thirty Road at Young Street, Young Street at Clayson Road, and Thirty Road at Clayson Road, is shown in **Figure 2.1**.

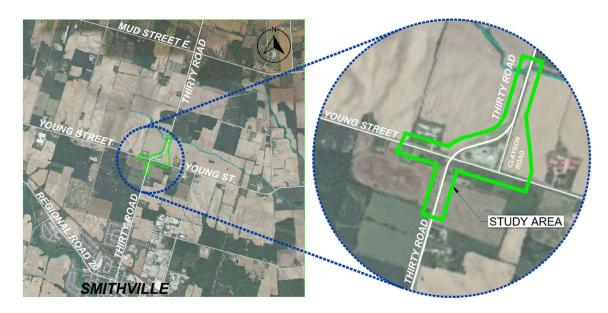


Figure 2.1 – Study Area

Thirty Road (RR14) is a two-lane rural arterial under the jurisdiction of the Region. As noted previously, Thirty Road presents a reverse curve north of Young Street, where it eventually aligns with Clayson Road. The roadway consists of a 26.2-metre right-of-way with a posted speed limit of 60km/hr south of Young Street and along the reverse curve, where it eventually transitions to 80km/hr north of the study area. Bike lanes are provided along Thirty Road, but there is no dedicated pedestrian accommodation in the vicinity of the study area.

Young Street is a two-lane rural arterial under the jurisdiction of the Township of West Lincoln. The corridor consists of a 20-metre right-of-way and a posted speed limit of 80km/hr. There is no infrastructure dedicated for pedestrians or cyclists in the vicinity of the study area.

Clayson Road is a two-lane rural corridor under the jurisdiction of the Township of West Lincoln. Approximately 200 metres in its entirety, the roadway is an additional link between Young Street and Thirty Road, and also provides access to the lands on the northeast corner of Thirty Road and Young Street. Due to the reverse curve on Thirty Road, the intersection of Clayson Road and Thirty Road exhibits a skewed intersection alignment. There is no active transportation infrastructure along the roadway.

2.2 Existing Lane Configurations

As noted in *Section 1.0*, the intersection of Thirty Road (RR14) and Young Street was previously controlled by a two-way stop with free-flow traffic along Thirty Road; however, the intersection has recently been converted to an all-way stop to address safety concerns. The intersection currently consists of four approaches, all of which provide a single lane with no turning restrictions.

The intersection of Young Street at Clayson Road is a three-leg intersection, with Young Street serving as the southern terminus for Clayson Road. The intersection consists of a single lane on each approach with free-flow traffic along Young Street and stop-control along Clayson Road.

The intersection of Thirty Road and Clayson Road exhibits a skewed alignment, as Clayson Road intersects near the end of the reverse curve along Thirty Road. While the skewed alignment makes it unsafe for turning movements to and from Clayson Road, there is no signage or infrastructure restricting the movements. The intersection consists of a single lane for each approach with stop control along Clayson Road.

The mode of control and lane configurations for the study area are presented in Figure 2.2.

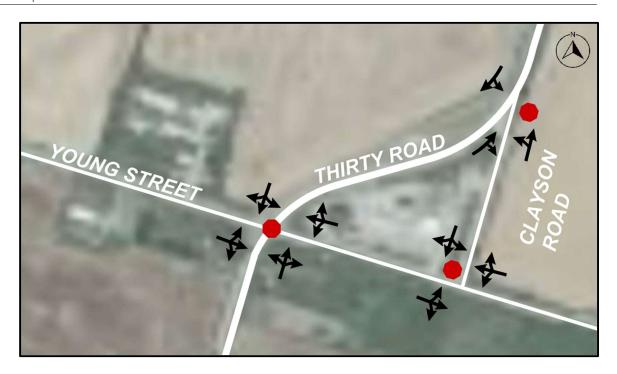


Figure 2.2 – Existing Intersection Configuration

2.3 Existing Traffic Volumes

Intersection turning movement count (TMC) data was collected on December 1st, 2022, for all three (3) intersections within the study area. Through analysis of the 8-hour counts for the entire study area, the morning and afternoon peak hours have been identified from 7:45am to 8:45am, and 4:15pm to 5:15pm, respectively.

In regard to active transportation users, the study area experiences nominal pedestrian activity, with the counts capturing only one pedestrian at the Thirty Road and Young Street intersection throughout the 8-hour period (outside of the peak hours). Furthermore, with the counts being completed in the winter, there were no cyclists recorded at any of the study area intersections. The morning and afternoon peak hour volumes are presented in **Figure 2.3**, with the full count data provided in **Appendix 1**.

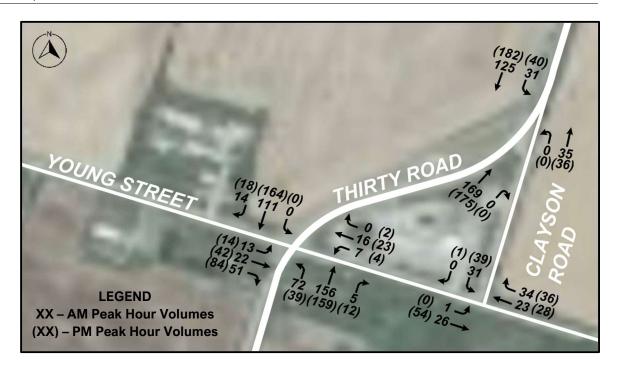


Figure 2.3 – Existing (2022) Traffic Volumes

2.4 Collision History (2017-2022)

The collision history at all three (3) study area intersections were reviewed as part of this study, inclusive of all collisions within a 5-year period of study commencement (Summer 2022). The earliest date recorded in this analysis occurred on August 3rd, 2017, and the most recent occurrence took place on June 29th, 2022. The collision data collected is presented in **Appendix 2**.

For Clayson Road & Young Street, a total of two (2) collisions took place in 2017, with one (1) involving a vehicle reversing into a stopped car, and the other involving a vehicle running off the road due to ice; neither collision resulted in a personal injury. Similarly, for Clayson Road and Thirty Road, one (1) collision took place in the 2017-2022 timeframe, with that collision involving a vehicle running off the road due to snow, with no personal injuries sustained as a result.

For Thirty Road & Young Street however, a total of 14 collisions took place in the 5-year period; the collision history is summarized in **Table 2.1**.

Number of Collisions % Distribution **Collision Type** 2017 2018 2019 2020 2021 2022 Total 2 3 Angle 2 1 3 0 11 79% 0 0 0 0 1 0 1 **Turning Movement** 7% Single Motor Vehicle 0 0 0 0 0 1 1 7% 0 0 0 0 1 7% Other 0 1 2 2 3 2 4 1 **Total** 14 100%

Table 2.1 – Thirty Road & Young Street Collision Summary (2017-2022)

As seen in the table, 79% of the collisions that have taken place at Thirty Road & Young Street in the most recent 5 years of collision data are classified as an angle collision, which is largely known to be one of the most severe collision types. Other collisions that have taken place at the intersection include a turning movement collision, a single motor vehicle collision, and a near-miss resulting in a vehicle running off the road to avoid a collision (classified as "other").

Further to the staggering percentage of angle collisions at the intersection, the review identified that eight (8) of the angle collisions included a westbound vehicle, six (6) of which were hit by vehicle travelling southbound along the spiral curve. It can be inferred from these numbers that sightline issues exist at this intersection for the westbound approach, largely due to the horizontal alignment of Thirty Road north of Young Street.

2.5 Traffic Operations Analysis Parameters

Intersection operations analysis has been completed for all the focus intersections using the industry standard macroscopic traffic analysis software, Synchro 11. The analysis adopts analysis parameters in accordance with the *Region of Niagara Guidelines for Transportation Impact Studies*, 2012; this information is summarized as follows:

Synchro

Saturation flow rate of 1750 was used for intersection analysis.

Critical Movements

Movements are to be deemed 'critical' when one or more of the following statements are true:

• Level of Service (LOS) based on average delay per vehicle, on individual movements exceeds LOS "D"; and

• The estimated 95th percentile queue length for an individual movement exceeds the available queue storage.

Key performance measures such as Level of Service (LOS), volume-to-capacity ratio (v/c ratio), and 95th percentile queuing was reported, and are defined below:

- Average vehicle control delay is used to characterize LOS for the entire intersection, an approach, or movement. Delay quantifies the variations in travel time and is also a surrogate measure of driver discomfort and fuel consumption.
- V/c ratio quantifies the degree to which the capacity of each signal phase is utilized by a defined lane group.
- 95th percentile queue is the queue length which is expected to be exceeded only 5% of the time; it is common practice to identify preferred storage length requirements for auxiliary turn lanes at signalized intersections based on estimated peak hour 95th percentile queueing.

The delay thresholds that dictate the LOS for unsignalized intersections, as per the Highway Capacity Manual (HCM 2000) methodology, are shown in **Table 2.2.**

Level of Service	Control Delay (seconds / vehicle)
(LOS)	Unsignalized Intersection
Α	≤ 10
В	> 10 to 15
С	> 15 to 25
D	> 25 to 35
Е	> 35 to 50
F	> 50

Table 2.2 – Characteristics of Level of Service at Unsignalized Intersections

2.6 Intersection Operations Analysis - Existing (2022) Traffic Conditions

Given the low traffic volumes at the study area intersections, an all-way stop warrant analysis for the intersection of Thirty Road and Young Street was performed and is presented in **Appendix 3**. The analysis concludes that all-way stop control is not warranted at this intersection using existing traffic volumes. However, due to the sightline concerns and safety issues identified in *Section 2.4 Collision History*, all-way stop control under existing conditions is warranted.

Traffic operations analysis has been undertaken for the existing (2022) traffic operations; the results of this analysis are summarized in **Table 2.3**. The detailed Synchro analysis outputs are provided in **Appendix 3**.

Peak Hour Weekday AM Weekday PM Movement Intersection Queue (m) Delay (s) Delay (s) 95% tile Traffic Control FOS FOS NC NC 1.4 0.30 1.2 0.32 Α 9.6 Α 9.6 **EBLTR WBLTR** 0.12 Α 8.1 0.4 0.20 Α 8.7 0.7 Thirty Road & Young Street **NBLTR** 0.04 Α 8.2 0.1 0.05 Α 8.4 0.1 All-Way Stop Control SBLTR 0.18 Α 8.6 0.6 0.26 Α 9.2 1.0 Α 9.0 Α 9.2 Intersection **EBTL** 0.00 Α 0.3 0.0 0.00 Α 0.0 0.0 **WBTR** 0.04 Α 0.0 0.0 0.04 Α 0.0 0.0 Young Street & Clayson Road 0.04 9.2 1.0 0.05 Α 9.3 1.2 SBLR Α Minor Stop Control Intersection Α 2.6 Α 2.3 0.05 Α 9.5 1.1 0.05 Α 9.5 1.2 **WBLR** 0.0 **NBTR** 0.11 Α 0.0 0.0 0.11 Α 0.0 Thirty Road & Clayson Road SBTL 0.02 Α 1.7 0.6 0.03 Α 1.6 8.0 Minor Stop Control Intersection Α 1.7 1.6

Table 2.3 – Existing (2022) Conditions - Traffic Operations Analysis Results

As seen in **Table 2.3**, the intersection of Thirty Road & Young Street operates with ample reserve capacity, delays at LOS "A" (less than 10 seconds of delay), and no queuing issues under the existing all-way stop control.

For the intersections of Thirty Road & Clayson Road, as well as Young Street & Clayson Road, both intersections are operating with substantial reserve capacity, nominal delays, and no queuing concerns under existing conditions.

2.7 Warrant Analysis – Existing Conditions

The left-turn lane warrant analysis for the northbound movement at Thirty Road and Young Street intersection was performed using the criteria outlined in Appendix 9A of the Ministry of Ontario Transportation (MTO) Design Supplement for Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads June 2017.

The analyses concludes that an exclusive left-turn lane for the northbound movement at Thirty Road and Young Street intersection is not required under the existing conditions. The warrant analysis is presented in **Appendix 4**.

3.0 Future (2041) Traffic Estimation and Planned Network Improvements

3.1 Traffic Growth Rate

A screenline analysis has been completed in the proximity of the intersection, north and south of Young Street, using link volumes from Regional EMME model output provided by the Region. These link volumes are summarized in **Table 3.1**, with the outputs provided in **Appendix 5**.

Table 3.1 - Traffic Growth Rates - EMME Model Data

			Tueffie			
Roadway Name	Movement	20	16	20	Traffic Growth	
Noadway Name	Weekday Weekd AM PM		Weekday PM	Weekday AM	Weekday PM	Rate
Thirty Dood (DD 14)	Northbound	494	474	814	846	2.3%
Thirty Road (RR 14)	Southbound	489	473	834	918	2.7%
Vouna Ctroot	Eastbound	8	11	12	18	2.0%
Young Street	Westbound	15	26	25	76	4.4%

Based on the results of the screenline analysis, in addition to consultation with Regional staff, a conservative growth rate of 2.5% has been adopted for the study area. The resulting 2041 traffic volumes are shown in **Figure 3.1**.

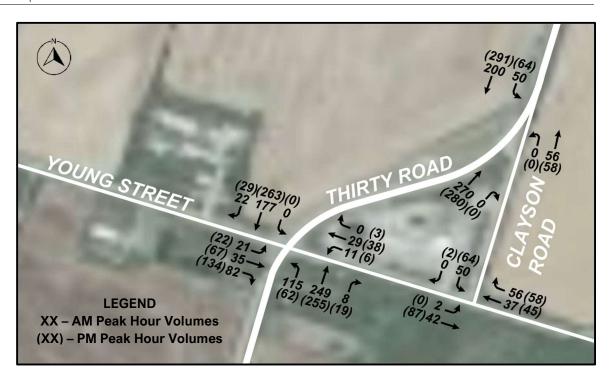


Figure 3.1 – Future (2041) Volumes (Annualized Growth Rate Only)

Given the alternatives include road closures, volume redistributions have been calculated accordingly for each alternative scenario; the projected traffic volumes adopted for each of the alternative scenarios are discussed under each improvement alternative.

3.2 Future Transportation Planned Network Improvements

As part of the *Smithville Transportation Master Plan (January 2023)*, a number of recommendations for new links and bypasses have been made for the Smithville area. Two (2) of these recommendations would have a substantial impact on the volumes in the study area, these two (2) recommendations are as follows:

- a. Implement a new connector road (classified as a collector or arterial) that travels east-west along the existing hydro corridor, from Industrial Park Road to Regional Road 20; and
- b. Provide a one (1) kilometre extension to Spring Creek Road that travels west and eventually curves north to intersect with the new connector road noted above.

The implementation of these corridors would provide alternative routes for east-west travel and would significantly reduce volumes along Young Street as a result. However, given the uncertainty of timelines for these projects, paired with the immediate need for operational

improvements within the study area, the analysis adopts the existing road network for future volume projections.

4.0 Future Traffic Operations - Improvement Alternatives

In consultation with the Regional staff, five (5) alternatives are being evaluated for the study area. The details of the alternative solutions considered are summarized in **Table 4.1**.

Table 4.1 – Summary of Alternative Solution

Alternative	Description
1 - Do Nothing	Maintain the existing alignment with the intersection operating as two-way stop control or all-way stop control
2 - Clayson Road Closure	Close Clayson Road and direct all traffic to an improved Thirty Road & Young Street intersection (i.e., traffic signal, roundabout).
3 - Thirty Road North Segment Closure	Close Thirty Road north of Young Street and provide two T-intersections with potential for improved traffic control (i.e., traffic signals, roundabouts).
4 - Realign Thirty Road north of Young Street	Close Clayson Road and realign Thirty Road, north of Young Street, to provide enhanced sightlines.
5 - Extend Clayson Road south of Young Street	Realign the south segment of Thirty Road to meet with Clayson Road.

An illustration and list of expected outcomes for each alternative is included in Appendix 6.

4.1 Forecast (2041) Traffic Volumes

The study adopts an ultimate horizon year of 2041 to align with the Region's EMME traffic forecast model, which is calibrated to the 2041 horizon year. Review of the Smithville TMP (2017) concluded that an extension of Industrial Park Road, as well as three (3) new corridors, named "New Western Link", "New Northern Connector", and "New Eastern Link", will be implemented as part of a by-pass through the Smithville downtown area. This by-pass has been recommended to mitigate cut-through traffic through the downtown Smithville area.

It was noted that the 2041 Emme model considers these additional links, which results in a significant decrease in traffic volumes along Young Street. However, given the by-pass links do not exist at the time of this study, a sensitivity analysis has been undertaken using the traffic volumes estimated in the 2041 Emme modelling, while still maintaining Young Street as a primary east-west corridor.

It should be noted that a cursory review of the current road network indicates that without the implementation of the "New Western Link" and "New Northern Connector", most drivers would utilize Townline Road to by-pass downtown Smithville, rather than Young Street. To maintain east-west volumes and align with the Region's 2041 traffic volume projections along Thirty Road, an annualized growth rate of 2.5% (as confirmed in *Section 3.1*) has been applied to all existing turning movements at the three (3) study area intersections, with additional through volumes along the Thirty Road corridor being taken from the Emme 2041 modelling south of Young Street. Given the Regional 2041 EMME model is inclusive of planned growth within the area, the model projections are considered to be the future total volumes, and are summarized in **Figure 4.1**.

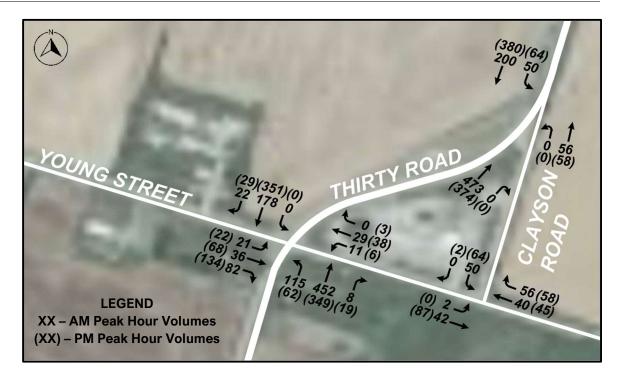


Figure 4.1 – Future (2041) Traffic Volumes – Do Nothing (Alternative #1)

It is concluded from the sensitivity analysis that the volumes shown in **Figure 4.1** are more representative of the Thirty Road and Young Street intersection, especially in the event that the implementation of the new by-pass links are delayed. As a result, these volumes have been carried forward for future analysis.

The future (2041) traffic volumes in each of the proposed improvement alternatives are estimated based on the corresponding road closures in each of the proposed improvement alternatives. It should be noted that while there are significant differences in geometric characteristics for Alternatives 2, 4, and 5, the traffic distribution will be similar for each option.

The estimated future (2041) traffic volumes are discussed in the following sections.

4.1.1 Alternative 1: Do Nothing

The future (2041) traffic volumes in the Do-Nothing scenario are presented in Figure 4.1.

4.1.2 Alternatives 2, 4, and 5: Clayson Road Closure, and Extend Clayson Road south of Young Street Closure

The future (2041) traffic volumes in Alternatives #2, #4, and #5 is presented in **Figure 4.2**. It is important to note that **Figure 4.2** reflects the Alternative #2 geometric alignment, and is

not reflective of the Alternative #4 and #5 geometric alignment. The figure should be referenced for traffic volumes only.

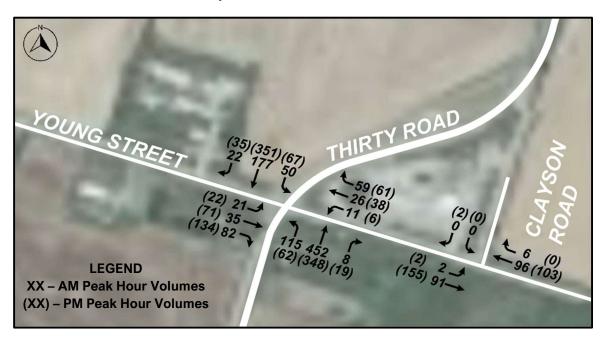


Figure 4.2 – Future (2041) Traffic Volumes – Clayson Road Closure (Alternatives #2 and 4) and Extend Clayson Road south of Young Street closure (Alternatives #5)

4.1.3 Alternatives 3: Thirty Road North Segment Closure

The future (2041) traffic volumes in Thirty Road north segment closure scenario is presented in **Figure 4.3**.

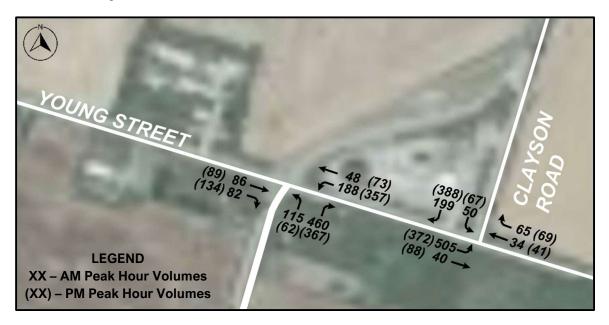


Figure 4.3 – Future (2041) Traffic Volumes – Thirty Road North Segment Closure (Alternatives #3)

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May 30, 2023

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4.2 Warrant Analysis – Future (2041) Traffic Conditions

Comparison of the future (2041) traffic volumes at the intersection of Thirty Road and Young Street among the proposed alternatives reveal that the Alternatives #2, #4, and #5 have the highest traffic volumes.

An all-way stop warrant analysis for the intersection of Thirty Road and Young Street was performed under the future (2041) traffic conditions and is presented in **Appendix 7**.

The left-turn lane warrant analysis for the northbound and southbound movements at Thirty Road and Young Street intersection was performed using the criteria outlined in Appendix 9A of the Ministry of Ontario Transportation (MTO) Design Supplement for Transportation Association of Canada (TAC) Geometric Design Guide for Canadian Roads June 2017.

The analyses concludes that an exclusive left-turn lane for the northbound and southbound movements at Thirty Road and Young Street intersection is required under the future (2041) traffic conditions with unsignalized control. The warrant analysis is presented in **Appendix 7**.

Signal warrant analysis for Thirty Road and Young Street intersection under future (2041) traffic conditions was analyzed using the guidelines provided under the Justification 7 (Projected Volumes) of Ontario Traffic Manual (OTM) Book 12. The analyses concludes that traffic signal is not required at Thirty Road and Young Street intersection under the future (2041) traffic conditions. The warrant analyses results are presented in **Appendix 7**.

4.3 Intersection Operations Analysis - Future (2041) Traffic Conditions

4.3.1 Alternative 1: Do Nothing

Traffic operations analysis has been undertaken for the Alternative 1 (Do Nothing) using Synchro software. The results of this analysis are summarized in **Table 4.2**. The detailed Synchro analysis outputs are provided in **Appendix 8**.

Table 4.2 – Future (2041) Conditions – Traffic Operations Analysis Results (Alternative 1 – Do Nothing)

					Peak	Hour			
	펕	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Veel	kday A	AΜ	Weekday PM			
Intersection Traffic Control	Movement	N/C	ros	Delay (s)	95% ^{tile} Queue (m)	N/C	FOS	Delay (s)	95% ^{tile} Queue (m)
Thirty Road & Young Street All-Way Stop Control	EBLTR WBLTR NBLTR SBLTR Intersection	0.24 0.08 0.86 0.33	B B D B	10.7 10.1 31.4 11.3 23.3	0.9 0.3 12.7 1.5	0.41 0.10 0.72 0.65	C B C C	22.0 13.2 10.8 18.3	6.1 2.0 0.3 4.6
Young Street & Clayson Road Minor Stop Control	EBTL WBTR SBLR Intersection	0.00 0.06 0.06	A A A	0.3 0.0 9.6 2.6	0.0 0.0 1.7	0.00 0.07 0.09	A A A	0.0 0.0 9.8 2.5	0.0 0.0 2.3
Thirty Road & Clayson Road Minor Stop Control	WBLR NBTR SBTL Intersection	0.11 0.30 0.06	B A A	12.5 0.0 2.2 1.6	3.0 0.0 1.4	0.10 0.24 0.06	B A A	11.3 0.0 1.8 1.7	2.6 0.0 1.6

As shown in **Table 4.2**, the intersection of Thirty Road & Clayson Road is forecast to exhibit overall intersection delays of LOS "C" (15 to 25 seconds of delay), with v/c ratios peaking at 0.86 for the eastbound movement in the a.m. peak hour. There are no queueing concerns anticipated through to the ultimate 2041 horizon year.

For the other two intersections along Clayson Road, both intersections are forecast to operate with reserve capacity, delays less than 15 seconds (LOS "B") for individual movements and no queueing concerns through to the ultimate 2041 horizon year.

4.3.2 Alternative 2: Close Clayson Road and Provide Traffic Control Improvements to Thirty Road & Young Street

Traffic operations analysis has been undertaken for the Alternative 2 (Clayson Closure) using Synchro software (for unsignalized traffic control intersections) and Arcady software (for roundabout). The results of this analysis are summarized in **Table 4.3**. The detailed Synchro and Arcady analyses outputs are provided in **Appendix 9**.

Thirty Road & Clayson Road

Roundabout Control

Peak Hour Weekday AM Weekday PM Movement Intersection Quene (m) **(s)** Delay (s) 95% tile Traffic Control FOS FOS Delay (NC NC 0.33 12.7 1.4 15.3 2.3 **EBLTR** В 0.45 С **WBLTR** 0.19 В 11.2 0.7 0.23 В 12.5 0.9 **Thirty Road & Young Street** 47.4 30.2 **NBLTR** 0.95 Ε 13.3 0.81 D 7.9 All-Way Stop Control 14.7 33.2 SBLTR 0.47 В 2.5 0.83 D 8.8

D

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31.4

7.5

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5.8

9.5

7.8

3.6

4.2

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21.0

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Α

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0.29

0.40

0.40

31.1

5.8

7.3

7.1

7.4

7.1

3.0

7.2

12.0

12.6

Intersection

EBLTR

WBLTR

NBLTR

SBLTR

Intersection

Table 4.3 - Future (2041) Conditions – Traffic Operations Analysis Results (Alternative 2 – Clayson Closure)

As shown in **Table 4.3**, the redistribution of traffic associated with closing Clayson Road results in capacity constraints for the all-way stop scenario; in particular, the increased traffic results in substantial delays for the northbound movement, with v/c ratios peaking at 0.95 in the a.m. peak hour.

For roundabout control, the intersection is forecast to operate with ample reserve capacity (v/c ratios not exceeding 0.53), delays less than 10 seconds (LOS "A"), and no queueing concerns. Furthermore, the conversion to roundabout control would serve to reduce angle collisions at this intersection.

4.3.3 Alternative 3: Close North Segment of Thirty Road and Create Two T-Intersections along Young Street

Traffic operations analysis has been undertaken for the Alternative 3 (Close north segment of Thirty Road and create two T-Intersections along Young Street) using Synchro software (for unsignalized traffic control intersections) and Arcady software (for roundabout). The results of this analysis are summarized in **Table 4.4**. The detailed Synchro and Arcady analysis outputs are provided in **Appendix 10**.

Table 4.4 - Future (2041) Conditions – Traffic Operations Analysis Results (Alternative 3 – Thirty Closure)

					Peak	Hour			
	펕	'	Wee	kday A	AM	'	Weel	kday P	M
Intersection Traffic Control	Movement	N/C	FOS	Delay (s)	95% ^{tile} Queue (m)	N/C	SOT	Delay (s)	95% ^{tile} Queue (m)
Thirty Road & Young Street (West Junction) Minor Stop Control	EBTR WBLT NBLR Intersection	0.11 0.15 0.88 -	A A E C	0.0 6.8 36.0 22.8	0.0 4.3 88.2	0.14 0.30 0.83	A A E C	0.0 7.9 35.2 17.1	0.0 10.0 68.2
Thirty Road & Young Street (East Junction) Minor Stop Control	EBTR WBLT SBLR Intersection	0.38 0.06 0.69	A A D	8.6 0.0 32.8 14.4	14.5 0.0 40.7	0.28 0.07 0.80	A A D	7.3 0.0 30.4 16.8	9.1 0.0 64.1
Thirty Road & Young Street (West Junction) Roundabout Control	EBTR WBLT NBLR Intersection	0.21 0.16 0.50	A A A	5.3 5.0 8.7 7.2	5.4 3.6 18.6	0.36 0.27 0.37	A A A	6.4 6.6 6.7 6.5	10.2 6.6 10.8
Thirty Road & Young Street (East Junction) Roundabout Control	EBTR WBLT SBLR Intersection	0.45 0.14 0.20	A A A	7.3 6.0 4.4 6.7	14.4 3.0 4.8	0.39 0.13 0.38	A A A	6.7 5.5 6.4 6.4	11.4 3.0 10.8

Table 4.4 shows that closing the north segment of Thirty Road and providing two T-intersections will result in significant delays (LOS "E") for the movements along Thirty Road when implementing stop control. In contrast, the implementation of roundabouts provides better levels of service, with all movements operating with reserve capacity and delays less than ten (10) seconds; however, despite the individual roundabouts operating well under 2041 traffic conditions, this alternative makes Thirty Road discontinuous and is therefore not desirable for a primary arterial.

4.3.4 Alternatives 4 & 5: Realign Thirty Road North/South of Young Street

Alternatives 4 and 5 consist of realigning Thirty Road to improve sightlines and create a more accommodating horizontal alignment, with Alternative 4 realigning Thirty Road north of Young Street and Alternative 5 realigning Thirty Road south of Young Street. From an operations analysis perspective, both of these options, as well as Alternative 3 (Clayson Road closure) result in the same volume distribution, and therefore yield the same results.

Given the volume scenarios do not meet the warrant thresholds for an all-way stop, in addition to the sightlines being greatly enhanced by the realignment of Thirty Road,

Alternative 4 and 5 include an analysis for two-way stop control, with free-flow movement along Thirty Road.

Traffic operations analysis has been undertaken for the Alternatives 4 and 5 (realign Thirty Road north/south of Young Street) using Synchro software (for unsignalized traffic control intersections) and Arcady software (for roundabout). The results of this analysis are summarized in **Table 4.5**. The detailed Synchro and Arcady analyses outputs are provided in **Appendix 9**.

Table 4.5 - Future (2041) Conditions – Traffic Operations Analysis Results (Alternatives 4 & 5 – Realigning Thirty Road North/South of Young Street)

					Peak	Hour			
	Έ	'	Wee	kday <i>l</i>	ΑM	Weekday PM			
Intersection Traffic Control	Movement	N/C	FOS	Delay (s)	95% ^{tile} Queue (m)	N/C	FOS	Delay (s)	95% ^{tile} Queue (m)
Thirty Road & Young Street Two-Way Stop Control	EBLTR WBLTR NBLTR SBLTR Intersection	0.76 0.38 0.09 0.05	F D A A	54.9 25.7 2.5 2.2 12.5	42.9 13.5 2.5 2.2	0.80 0.41 0.06 0.06	F D A A B	51.5 26.9 1.8 1.8 13.2	52.1 15.4 1.5 1.6
Thirty Road & Young Street All-Way Stop Control	EBLTR WBLTR NBLTR SBLTR Intersection	0.33 0.19 0.95 0.47	B B E B	12.7 11.2 47.4 14.7 31.4	1.4 0.7 13.3 2.5	0.45 0.23 0.81 0.83	C B D D	15.3 12.5 30.2 33.2 31.1	2.3 0.9 7.9 8.8
Thirty Road & Clayson Road Roundabout Control	EBLTR WBLTR NBLTR SBLTR Intersection	0.17 0.27 0.19 0.57	A A A A	7.3 5.7 5.4 9.9 8.0	3.6 6.6 4.2 22.2	0.14 0.42 0.29 0.42	A A A A	5.7 7.2 7.4 7.5 7.2	3.0 12.6 7.2 12.6

As shown in **Table 4.5**, the realigned Thirty Road and Young Street intersection is forecast to operate with significant delays (LOS "F") for the minor approach movements under two-way stop control. For all-way stop control, overall intersection delays are increased to LOS "D" (25 to 35 seconds of delay) for both peak periods, with v/c ratios peaking at 0.95 for the northbound movement.

Under roundabout configuration, the intersection operates with substantial reserve capacity, delays less than 10 seconds (LOS "A"), and no queuing concerns. Therefore, the roundabout control is forecast to provide the most desirable performance measures under the realigned Thirty Road alternatives.

5.0 Summary of Findings and Recommendations

The findings and recommendations from the transportation assessment can be summarized as follows:

- The intersection of Thirty Road & Young Street has been reviewed in response to significant resident concern regarding traffic safety, largely due to the spiral curve along Thirty Road, north of Young Street.
- A review of collision history from 2017-2022 identified that 14 collisions have taken place, 11 of which were classified as angle collisions, which are largely known to be one of the most severe collision types.
- The collision history showed that 8 of the 11 angle collisions included a westbound vehicle, 6 of which also included a southbound vehicle, which infers that a sightline issue exists on the westbound and southbound approaches due to the horizontal alignment along Thirty Road, north of Young Street.
- Future (2041) intersection volumes were developed by undertaking a sensitivity analysis using the estimated growth rate of 2.5% applied to the existing (2022) traffic volumes, in addition to using information from the Smithville TMP (2017) and 2041 Emme model outputs.
- OTM warrant analyses for all-way stop control and signals have been undertaken for the Thirty Road and Young Street intersection, with the results indicating that neither all-way stop controls, nor traffic signals, are warranted at this intersection under existing or future volume scenarios. However, the collision analysis concluded that all-way stop control is warranted due to sightline issues.
- The all-way stop control scenarios are anticipated to cause some additional delays for traffic along Thirty Road.
- The roundabout configuration is forecast to operate with substantial reserve capacity, nominal delays, and no queueing concerns. Furthermore, the roundabout configuration will serve to reduce angle collisions at Thirty Road & Young Street.
- For traffic accommodation, it is recommended that Improvement Alternatives 3, 4 or 5 under roundabout control be implemented, as these alternatives under roundabout control maintain the arterial functionality of Thirty Road while also servicing the minor volumes along Young Street.

APPENDIX 1

Turning Movement Count (TMC) Data





Project #22-410 - RV Anderson

Intersection Count Report

Intersection: Thirty Rd & Young St

Municipality: Smithville

Count Date: Thursday, Dec 01, 2022

Site Code: 2241000002

Count Categories: Cars, Trucks, Bicycles, Pedestrians

Count Period: 07:00-09:00, 11:00-14:00, 15:00-18:00

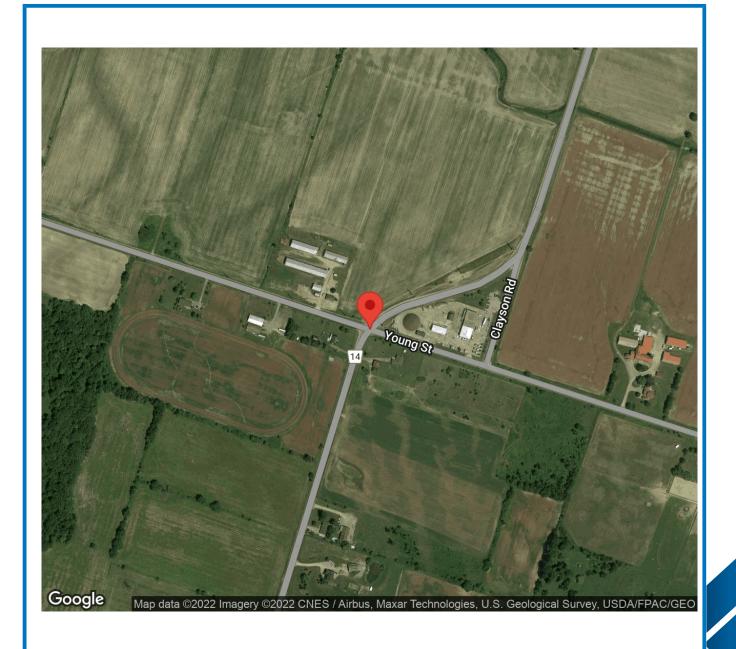
Weather: Clear

Comments:



Traffic Count Map

Intersection: Thirty Rd & Young St
Site Code: 2241000002
Municipality: Smithville
Count Date: Dec 01, 2022





Traffic Count Summary

Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

Thirty Rd - Traffic Summary

		North	Appr	oach T	otals								
		Include	s Cars, 1	Trucks, Bi	cycles								
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Tota
07:00 - 08:00	0	75	6	0	81	0	77	139	6	0	222	0	303
08:00 - 09:00	0	94	18	0	112	0	63	151	3	0	217	0	329
					В	REAK							
11:00 - 12:00	0	74	7	0	81	1	49	103	3	0	155	0	236
12:00 - 13:00	0	91	7	0	98	0	40	86	6	0	132	0	230
13:00 - 14:00	0	76	8	0	84	0	35	84	4	0	123	0	207
					В	REAK							
15:00 - 16:00	0	131	15	0	146	0	58	140	3	0	201	0	347
16:00 - 17:00	1	162	15	0	178	0	41	153	11	0	205	0	383
17:00 - 18:00	0	144	15	0	159	0	31	103	4	0	138	0	297
GRAND TOTAL	1	847	91	0	939	1	394	959	40	0	1393	0	2332



Traffic Count Summary

Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

Young St - Traffic Summary

		East	Appro	ach To	tals								
		Include	s Cars, 1	Trucks, Bi	icycles		Includes Cars, Trucks, Bicycles						
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	6	23	1	0	30	0	16	23	48	0	87	0	117
08:00 - 09:00	5	15	0	0	20	0	12	27	53	1	93	0	113
	BREAK												
11:00 - 12:00	2	11	1	0	14	0	10	17	36	0	63	0	77
12:00 - 13:00	6	12	2	0	20	0	6	20	54	0	80	0	100
13:00 - 14:00	6	18	3	0	27	0	12	26	48	0	86	0	113
					В	REAK							
15:00 - 16:00	5	30	0	0	35	0	12	19	83	0	114	0	149
16:00 - 17:00	5	24	2	0	31	0	13	37	94	0	144	0	175
17:00 - 18:00	2	17	1	0	20	0	13	35	62	0	110	0	130
GRAND TOTAL	37	150	10	0	197	0	94	204	478	1	777	0	974



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

	Cars					Bicycles										
Start Time	4	1	•	1	Total	4	1	•	J.	Total	4	1	•	1	Total	Total Peds
07:00	0	8	1	0	9	0	0	0	0	0	0	0	0	0	0	0
07:15	0	16	1	0	17	0	2	0	0	2	0	0	0	0	0	0
07:30	0	16	2	0	18	0	2	0	0	2	0	0	0	0	0	0
07:45	0	28	2	0	30	0	3	0	0	3	0	0	0	0	0	0
08:00	0	22	4	0	26	0	3	0	0	3	0	0	0	0	0	0
08:15	0	27	3	0	30	0	4	0	0	4	0	0	0	0	0	0
08:30	0	19	4	0	23	0	5	1	0	6	0	0	0	0	0	0
08:45	0	14	3	0	17	0	0	3	0	3	0	0	0	0	0	0
SUBTOTAL	0	150	20	0	170	0	19	4	0	23	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

	Cars						Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	0	19	0	0	19	0	1	0	0	1	0	0	0	0	0	0
11:15	0	13	2	0	15	0	3	0	0	3	0	0	0	0	0	0
11:30	0	14	1	0	15	0	3	1	0	4	0	0	0	0	0	0
11:45	0	18	3	0	21	0	3	0	0	3	0	0	0	0	0	1
12:00	0	17	1	0	18	0	2	0	0	2	0	0	0	0	0	0
12:15	0	22	0	0	22	0	0	0	0	0	0	0	0	0	0	0
12:30	0	27	1	0	28	0	3	0	0	3	0	0	0	0	0	0
12:45	0	19	5	0	24	0	1	0	0	1	0	0	0	0	0	0
13:00	0	14	1	0	15	0	4	0	0	4	0	0	0	0	0	0
13:15	0	18	1	0	19	0	0	1	0	1	0	0	0	0	0	0
13:30	0	17	4	0	21	0	0	0	0	0	0	0	0	0	0	0
13:45	0	23	1	0	24	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	221	20	0	241	0	20	2	0	22	0	0	0	0	0	1



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	0	15	4	0	19	0	1	1	0	2	0	0	0	0	0	0
15:15	0	40	2	0	42	0	2	1	0	3	0	0	0	0	0	0
15:30	0	34	3	0	37	0	2	0	0	2	0	0	0	0	0	0
15:45	0	35	4	0	39	0	2	0	0	2	0	0	0	0	0	0
16:00	1	34	3	0	38	0	2	0	0	2	0	0	0	0	0	0
16:15	0	35	2	0	37	0	0	1	0	1	0	0	0	0	0	0
16:30	0	41	3	0	44	0	3	0	0	3	0	0	0	0	0	0
16:45	0	45	6	0	51	0	2	0	0	2	0	0	0	0	0	0
17:00	0	35	6	0	41	0	2	0	0	2	0	0	0	0	0	0
17:15	0	40	2	0	42	0	1	0	0	1	0	0	0	0	0	0
17:30	0	40	3	0	43	0	3	0	0	3	0	0	0	0	0	0
17:45	0	22	4	0	26	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	1	416	42	0	459	0	21	3	0	24	0	0	0	0	0	0
GRAND TOTAL	1	787	82	0	870	0	60	9	0	69	0	0	0	0	0	1



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

South Approach - Thirty Rd

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	11	38	1	0	50	0	0	0	0	0	0	0	0	0	0	0
07:15	12	31	0	0	43	0	3	1	0	4	0	0	0	0	0	0
07:30	29	31	0	0	60	1	3	1	0	5	0	0	0	0	0	0
07:45	22	29	3	0	54	2	4	0	0	6	0	0	0	0	0	0
08:00	11	35	1	0	47	2	6	0	0	8	0	0	0	0	0	0
08:15	18	36	1	0	55	2	3	0	0	5	0	0	0	0	0	0
08:30	15	42	0	0	57	0	1	0	0	1	0	0	0	0	0	0
08:45	15	27	1	0	43	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	133	269	7	0	409	7	21	2	0	30	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

South Approach - Thirty Rd

			Cars				Tı	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	13	25	1	0	39	0	1	0	0	1	0	0	0	0	0	0
11:15	8	27	0	0	35	0	2	0	0	2	0	0	0	0	0	0
11:30	16	30	1	0	47	1	0	0	0	1	0	0	0	0	0	0
11:45	9	15	0	0	24	2	3	1	0	6	0	0	0	0	0	0
12:00	9	16	1	0	26	0	1	0	0	1	0	0	0	0	0	0
12:15	9	19	0	0	28	0	4	0	0	4	0	0	0	0	0	0
12:30	12	19	1	0	32	0	1	1	0	2	0	0	0	0	0	0
12:45	8	24	2	0	34	2	2	1	0	5	0	0	0	0	0	0
13:00	8	19	0	0	27	1	1	0	0	2	0	0	0	0	0	0
13:15	7	13	2	0	22	0	2	0	0	2	0	0	0	0	0	0
13:30	11	21	0	0	32	0	1	1	0	2	0	0	0	0	0	0
13:45	8	25	1	0	34	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	118	253	9	0	380	6	20	4	0	30	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

South Approach - Thirty Rd

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	13	42	1	0	56	0	0	0	0	0	0	0	0	0	0	0
15:15	15	34	0	0	49	2	1	0	0	3	0	0	0	0	0	0
15:30	16	29	0	0	45	0	1	1	0	2	0	0	0	0	0	0
15:45	12	31	0	0	43	0	2	1	0	3	0	0	0	0	0	0
16:00	10	28	1	0	39	0	0	0	0	0	0	0	0	0	0	0
16:15	11	43	1	0	55	0	1	2	0	3	0	0	0	0	0	0
16:30	10	47	5	0	62	1	2	0	0	3	0	0	0	0	0	0
16:45	9	32	2	0	43	0	0	0	0	0	0	0	0	0	0	0
17:00	7	31	2	0	40	1	2	0	0	3	0	0	0	0	0	0
17:15	9	30	0	0	39	0	1	0	0	1	0	0	0	0	0	0
17:30	9	16	1	0	26	0	1	0	0	1	0	0	0	0	0	0
17:45	5	22	1	0	28	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	126	385	14	0	525	4	11	4	0	19	0	0	0	0	0	0
GRAND TOTAL	377	907	30	0	1314	17	52	10	0	79	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				Tı	rucks				Bi	cycles			
Start Time	•	1	P	1	Total	4	1	•	1	Total	4	1	P	1	Total	Total Peds
07:00	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
07:15	3	3	0	0	6	0	0	0	0	0	0	0	0	0	0	0
07:30	0	8	1	0	9	1	3	0	0	4	0	0	0	0	0	0
07:45	2	5	0	0	7	0	1	0	0	1	0	0	0	0	0	0
08:00	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
08:15	3	2	0	0	5	0	0	0	0	0	0	0	0	0	0	0
08:30	2	5	0	0	7	0	1	0	0	1	0	0	0	0	0	0
08:45	0	5	0	0	5	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	10	31	1	0	42	1	7	0	0	8	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	1	4	0	0	5	0	0	0	0	0	0	0	0	0	0	0
11:15	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
11:30	0	3	1	0	4	1	0	0	0	1	0	0	0	0	0	0
11:45	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
12:00	0	4	0	0	4	1	0	0	0	1	0	0	0	0	0	0
12:15	2	4	0	0	6	0	1	0	0	1	0	0	0	0	0	0
12:30	1	1	0	0	2	1	0	1	0	2	0	0	0	0	0	0
12:45	0	1	1	0	2	1	1	0	0	2	0	0	0	0	0	0
13:00	2	3	0	0	5	1	2	0	0	3	0	0	0	0	0	0
13:15	0	3	0	0	3	0	2	1	0	3	0	0	0	0	0	0
13:30	0	3	0	0	3	1	1	0	0	2	0	0	0	0	0	0
13:45	0	4	2	0	6	2	0	0	0	2	0	0	0	0	0	0
SUBTOTAL	6	34	4	0	44	8	7	2	0	17	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	1	10	0	0	11	0	0	0	0	0	0	0	0	0	0	0
15:15	1	3	0	0	4	0	1	0	0	1	0	0	0	0	0	0
15:30	1	5	0	0	6	1	3	0	0	4	0	0	0	0	0	0
15:45	0	8	0	0	8	1	0	0	0	1	0	0	0	0	0	0
16:00	1	6	0	0	7	0	0	0	0	0	0	0	0	0	0	0
16:15	1	5	0	0	6	0	1	0	0	1	0	0	0	0	0	0
16:30	1	5	0	0	6	0	0	0	0	0	0	0	0	0	0	0
16:45	2	7	2	0	11	0	0	0	0	0	0	0	0	0	0	0
17:00	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0
17:15	2	8	0	0	10	0	0	0	0	0	0	0	0	0	0	0
17:30	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
17:45	0	3	1	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	10	66	3	0	79	2	5	0	0	7	0	0	0	0	0	0
GRAND TOTAL	26	131	8	0	165	11	19	2	0	32	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				Ti	rucks				Bi	cycles			
Start Time	4	1	P	1	Total	4	1	•	1	Total	4	1	P	1	Total	Total Peds
07:00	3	5	5	0	13	0	1	0	0	1	0	0	0	0	0	0
07:15	8	5	11	0	24	0	1	1	0	2	0	0	0	0	0	0
07:30	0	6	10	0	16	0	1	1	0	2	0	0	0	0	0	0
07:45	5	4	19	0	28	0	0	1	0	1	0	0	0	0	0	0
08:00	3	4	4	0	11	0	2	2	0	4	0	0	0	0	0	0
08:15	1	3	13	0	17	0	0	1	0	1	0	0	0	0	0	0
08:30	4	8	11	0	23	0	1	0	0	1	0	0	0	0	0	0
08:45	3	8	20	0	31	1	1	2	1	5	0	0	0	0	0	0
SUBTOTAL	27	43	93	0	163	1	7	8	1	17	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	4	1	8	0	13	1	2	1	0	4	0	0	0	0	0	0
11:15	2	4	7	0	13	0	1	0	0	1	0	0	0	0	0	0
11:30	0	6	10	0	16	0	0	0	0	0	0	0	0	0	0	0
11:45	1	2	8	0	11	2	1	2	0	5	0	0	0	0	0	0
12:00	3	2	16	0	21	1	1	1	0	3	0	0	0	0	0	0
12:15	0	3	10	0	13	0	0	0	0	0	0	0	0	0	0	0
12:30	1	5	11	0	17	0	0	0	0	0	0	0	0	0	0	0
12:45	1	8	14	0	23	0	1	2	0	3	0	0	0	0	0	0
13:00	2	3	13	0	18	0	1	0	0	1	0	0	0	0	0	0
13:15	3	7	11	0	21	0	1	1	0	2	0	0	0	0	0	0
13:30	1	3	9	0	13	0	2	1	0	3	0	0	0	0	0	0
13:45	4	8	12	0	24	2	1	1	0	4	0	0	0	0	0	0
SUBTOTAL	22	52	129	0	203	6	11	9	0	26	0	0	0	0	0	0



Intersection: Thirty Rd & Young St

Site Code: 2241000002 Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				Ti	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	3	4	20	0	27	0	2	1	0	3	0	0	0	0	0	0
15:15	1	2	14	0	17	0	1	1	0	2	0	0	0	0	0	0
15:30	2	4	25	0	31	1	2	0	0	3	0	0	0	0	0	0
15:45	5	4	22	0	31	0	0	0	0	0	0	0	0	0	0	0
16:00	2	8	31	0	41	0	0	1	0	1	0	0	0	0	0	0
16:15	4	9	21	0	34	0	0	0	0	0	0	0	0	0	0	0
16:30	5	8	21	0	34	0	0	0	0	0	0	0	0	0	0	0
16:45	2	12	20	0	34	0	0	0	0	0	0	0	0	0	0	0
17:00	3	13	21	0	37	0	0	1	0	1	0	0	0	0	0	0
17:15	5	5	12	0	22	0	0	3	0	3	0	0	0	0	0	0
17:30	2	9	12	0	23	0	0	2	0	2	0	0	0	0	0	0
17:45	3	8	10	0	21	0	0	1	0	1	0	0	0	0	0	0
SUBTOTAL	37	86	229	0	352	1	5	10	0	16	0	0	0	0	0	0
GRAND TOTAL	86	181	451	0	718	8	23	27	1	59	0	0	0	0	0	0



Peak Hour Diagram

Specified Period

One Hour Peak

From:

To:

From: To:

07:00:00 09:00:00 07:45:00 08:45:00

Intersection: Thirty Rd & Young St

Site Code: Count Date: 2241000002 Dec 01, 2022 Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	109	155	264
	16	14	30
<i>₫</i>	0	0	0
	125	169	294

Thirty Rd

	48	1	L	Ú
Totals	14	111	0	0
	13	96	0	0
	1	15	0	0
<i>₫</i> €	0	0	0	0

East Approach

	Out	In	Total
	19	24	43
	3	3	6
ॐ	0	0	0
	22	27	49

Young St

	Totals			₫	
7	0	0	0	0	
4	13	13	0	0	
\Rightarrow	22	19	3	0	
4	51	47	4	0	

Peds: 0



Young St

	Totals			<i>₫</i>
C	0	0	0	0
£	0	0	0	0
-	15	12	3	0
F	7	7	0	0

West Approach

	Out	In	Total
	79	91	170
	7	10	17
<i>₹</i>	0	0	0
	86	101	187

	4	1		J.
Totals	72	156	5	0
	66	142	5	0
	6	14	0	0
<i>₹</i>	0	0	0	0

Peds: 0

Thirty Rd

South Approach

	Out	In	Total
	213	150	363
	20	19	39
<i>₫</i>	0	0	0
	233	169	402







Comments



Peak Hour Summary

Intersection: Thirty Rd & Young St

 Site Code:
 2241000002

 Count Date:
 Dec 01, 2022

Period: 07:00 - 09:00

Peak Hour Data (07:45 - 08:45)

	North Approach Thirty Rd						South Approach Thirty Rd							East Approach Young St						West Approach Young St					
Start Time	4	1	P	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	1	Peds	Total	Vehicl es
07:45	0	31	2	0	0	33	24	33	3	0	0	60	2	6	0	0	0	8	5	4	20	0	0	29	130
08:00	0	25	4	0	0	29	13	41	1	0	0	55	0	1	0	0	0	1	3	6	6	0	0	15	100
08:15	0	31	3	0	0	34	20	39	1	0	0	60	3	2	0	0	0	5	1	3	14	0	0	18	117
08:30	0	24	5	0	0	29	15	43	0	0	0	58	2	6	0	0	0	8	4	9	11	0	0	24	119
Grand Total	0	111	14	0	0	125	72	156	5	0	0	233	7	15	0	0	0	22	13	22	51	0	0	86	466
Approach %	0	88.8	11.2	0		-	30.9	67	2.1	0		-	31.8	68.2	0	0		-	15.1	25.6	59.3	0		-	
Totals %	0	23.8	3	0		26.8	15.5	33.5	1.1	0		50	1.5	3.2	0	0		4.7	2.8	4.7	10.9	0		18.5	
PHF	0	0.9	0.7	0		0.92	0.75	0.91	0.42	0		0.97	0.58	0.63	0	0		0.69	0.65	0.61	0.64	0		0.74	0.9
Cars	0	96	13	0		109	66	142	5	0		213	7	12	0	0		19	13	19	47	0		79	420
% Cars	0	86.5	92.9	0		87.2	91.7	91	100	0		91.4	100	80	0	0		86.4	100	86.4	92.2	0		91.9	90.1
Trucks	0	15	1	0		16	6	14	0	0		20	0	3	0	0		3	0	3	4	0		7	46
% Trucks	0	13.5	7.1	0		12.8	8.3	9	0	0		8.6	0	20	0	0		13.6	0	13.6	7.8	0		8.1	9.9
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 11:00:00 14:00:00

From: 12:00:00 To: 13:00:00

Intersection: Thirty Rd & Young St

 Site Code:
 2241000002

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	92	84	176
	6	10	16
<i>₹</i>	0	0	0
	98	94	192

Thirty Rd

	48	1	L	Ĵ
Totals	7	91	0	0
	7	85	0	0
	0	6	0	0
₫	0	0	0	0

Peds: 0

East Approach

	Out	In	Total
	14	22	36
	6	4	10
₫ %	0	0	0
	20	26	46

Young St

ls	Totals			₫
0 💙	0	0	0	0
6 👍	6	5	1	0
.0 →	20	18	2	0
4 🖫	54	51	3	0





Young St

	Totals			<i>₫</i> %
C	0	0	0	0
Ł	2	1	1	0
-	12	10	2	0
F	6	3	3	0

West Approach

	Out	In	Total
	74	55	129
	6	4	10
<i>₹</i>	0	0	0
	80	59	139

	4	1		J.
Totals	40	86	6	0
	38	78	4	0
₽	2	8	2	0
<i>₹</i>	0	0	0	0

Peds: 0

Thirty Rd

South Approach

	Out	In	Total
	120	139	259
	12	12	24
₫ %	0	0	0
	132	151	283



🚨 - Trucks

- Bicycles

Comments



Peak Hour Summary

Intersection: Thirty Rd & Young St

 Site Code:
 2241000002

 Count Date:
 Dec 01, 2022

Period: 11:00 - 14:00

Peak Hour Data (12:00 - 13:00)

	North Approach Thirty Rd						South Approach Thirty Rd							East Approach Young St						West Approach Young St					
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	P	J	Peds	Total	es
12:00	0	19	1	0	0	20	9	17	1	0	0	27	1	4	0	0	0	5	4	3	17	0	0	24	76
12:15	0	22	0	0	0	22	9	23	0	0	0	32	2	5	0	0	0	7	0	3	10	0	0	13	74
12:30	0	30	1	0	0	31	12	20	2	0	0	34	2	1	1	0	0	4	1	5	11	0	0	17	86
12:45	0	20	5	0	0	25	10	26	3	0	0	39	1	2	1	0	0	4	1	9	16	0	0	26	94
Grand Total	0	91	7	0	0	98	40	86	6	0	0	132	6	12	2	0	0	20	6	20	54	0	0	80	330
Approach %	0	92.9	7.1	0		-	30.3	65.2	4.5	0		-	30	60	10	0		-	7.5	25	67.5	0		-	
Totals %	0	27.6	2.1	0	,	29.7	12.1	26.1	1.8	0	,	40	1.8	3.6	0.6	0	,	6.1	1.8	6.1	16.4	0		24.2	
PHF	0	0.76	0.35	0		0.79	0.83	0.83	0.5	0		0.85	0.75	0.6	0.5	0		0.71	0.38	0.56	0.79	0		0.77	0.88
Cars	0	85	7	0		92	38	78	4	0		120	3	10	1	0		14	5	18	51	0		74	300
% Cars	0	93.4	100	0		93.9	95	90.7	66.7	0		90.9	50	83.3	50	0		70	83.3	90	94.4	0		92.5	90.9
Trucks	0	6	0	0		6	2	8	2	0		12	3	2	1	0		6	1	2	3	0		6	30
% Trucks	0	6.6	0	0		6.1	5	9.3	33.3	0		9.1	50	16.7	50	0		30	16.7	10	5.6	0		7.5	9.1
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 15:00:00 To: 18:00:00 From: 16:15:00 To: 17:15:00

Intersection: Thirty Rd & Young St Site Code: 2241000002

Count Date: Dec 01, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	173	169	342
	8	5	13
<i>₹</i>	0	0	0
	181	174	355

Thirty Rd

	4	1	L	Ĵ
Totals	18	163	0	0
⊟	17	156	0	0
다	1	7	0	0
₫	0	0	0	0

East Approach

	Out	In	Total
	28	52	80
	1	2	3
₫ %	0	0	0
	29	54	83

Young St

	Totals			<i>₫</i>
7	0	0	0	0
4	14	14	0	0
\rightarrow	42	42	0	0
4	84	83	1	0



Peds: 0



Peds: 0

Young St

	Totals			₫ %
C	0	0	0	0
Ł	2	2	0	0
(-	23	22	1	0
F	4	4	0	0

West Approach

	Out	In	Total
	139	76	215
	1	4	5
<i>₫</i> 6	0	0	0
	140	80	220

	4	1		J
Totals	39	158	12	0
	37	153	10	0
	2	5	2	0
<i>₫</i> %	0	0	0	0

Thirty Rd

South Approach

	Out	In	Total
	200	243	443
	9	8	17
ॐ	0	0	0
	209	251	460







Comments



Peak Hour Summary

Intersection: Thirty Rd & Young St

 Site Code:
 2241000002

 Count Date:
 Dec 01, 2022

Period: 15:00 - 18:00

Peak Hour Data (16:15 - 17:15)

		N	North A Thirt	pproac y Rd	h			S	outh A Thir	pproac ty Rd	h				East Ap You	proach ng St	1		West Approach Young St					Total Vehicl	
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	•	1	P	J	Peds	Total	4	1	•	J	Peds	Total	es
16:15	0	35	3	0	0	38	11	44	3	0	0	58	1	6	0	0	0	7	4	9	21	0	0	34	137
16:30	0	44	3	0	0	47	11	49	5	0	0	65	1	5	0	0	0	6	5	8	21	0	0	34	152
16:45	0	47	6	0	0	53	9	32	2	0	0	43	2	7	2	0	0	11	2	12	20	0	0	34	141
17:00	0	37	6	0	0	43	8	33	2	0	0	43	0	5	0	0	0	5	3	13	22	0	0	38	129
Grand Total	0	163	18	0	0	181	39	158	12	0	0	209	4	23	2	0	0	29	14	42	84	0	0	140	559
Approach %	0	90.1	9.9	0		-	18.7	75.6	5.7	0		-	13.8	79.3	6.9	0		-	10	30	60	0		-	
Totals %	0	29.2	3.2	0		32.4	7	28.3	2.1	0		37.4	0.7	4.1	0.4	0		5.2	2.5	7.5	15	0		25	
PHF	0	0.87	0.75	0		0.85	0.89	0.81	0.6	0		8.0	0.5	0.82	0.25	0		0.66	0.7	0.81	0.95	0		0.92	0.92
Cars	0	156	17	0		173	37	153	10	0		200	4	22	2	0		28	14	42	83	0		139	540
% Cars	0	95.7	94.4	0		95.6	94.9	96.8	83.3	0		95.7	100	95.7	100	0		96.6	100	100	98.8	0		99.3	96.6
Trucks	0	7	1	0		8	2	5	2	0		9	0	1	0	0		1	0	0	1	0		1	19
% Trucks	0	4.3	5.6	0		4.4	5.1	3.2	16.7	0		4.3	0	4.3	0	0		3.4	0	0	1.2	0		0.7	3.4
Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Project #22-410 - RV Anderson

Intersection Count Report

Intersection: Thirty Rd & Clayson Rd

Municipality: Smithville

Count Date: Thursday, Dec 01, 2022

Site Code: 2241000001

Count Categories: Cars, Trucks, Bicycles, Pedestrians

Count Period: 07:00-09:00, 11:00-14:00, 15:00-18:00

Weather: Clear

Comments:



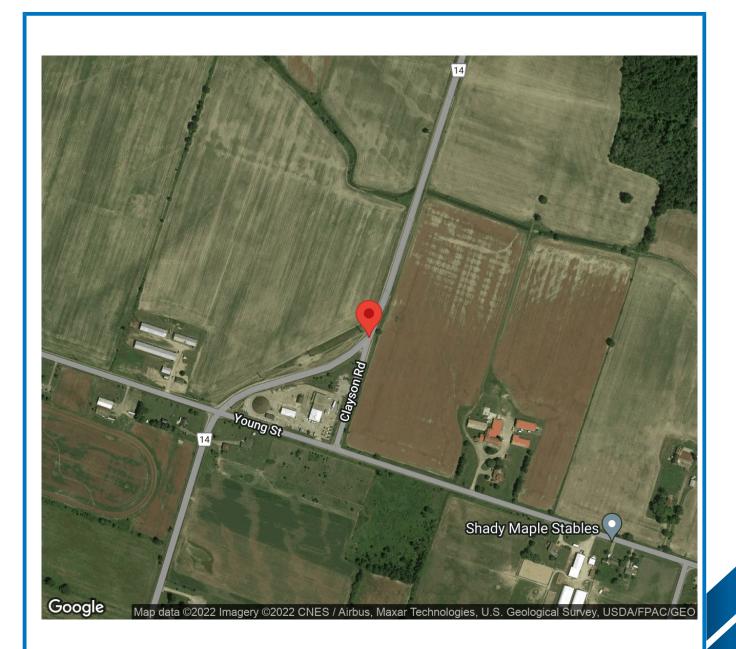
Traffic Count Map

Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022





Traffic Count Summary

Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001 Municipality: Smithville

Count Date: Dec 01, 2022

Thirty Rd - Traffic Summary

		North	Appr	oach T	otals								
		Include	s Cars, 1	Trucks, B	icycles			Include	s Cars, 1	rucks, B	icycles		
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	29	80	0	0	109	0	0	154	0	0	154	0	263
08:00 - 09:00	25	111	0	0	136	0	0	165	0	0	165	0	301
	BREAK												
11:00 - 12:00	32	81	0	0	113	0	0	113	1	0	114	0	227
12:00 - 13:00	38	95	0	0	133	0	0	92	0	0	92	0	225
13:00 - 14:00	32	83	0	0	115	0	0	100	1	0	101	0	216
					В	REAK							
15:00 - 16:00	54	145	0	0	199	0	0	152	0	0	152	0	35
16:00 - 17:00	51	178	0	0	229	0	0	169	0	0	169	0	398
17:00 - 18:00	36	159	0	0	195	0	0	116	0	0	116	0	31
GRAND TOTAL	297	932	0	0	1229	0	0	1061	2	0	1063	0	2292



Traffic Count Summary

Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001 Municipality: Smithville

Count Date: Dec 01, 2022

Clayson Rd - Traffic Summary

		East /	Appro	ach To	tals		West Approach Totals						
		Include	s Cars, 1	Trucks, Bi	cycles		Includes Cars, Trucks, Bicycles						
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	1	0	45	0	46	0	0	0	0	0	0	0	46
08:00 - 09:00	1	0	35	0	36	0	0	0	0	0	0	0	36
BREAK													
11:00 - 12:00	0	0	21	0	21	0	0	0	0	0	0	0	21
12:00 - 13:00	3	0	25	0	28	0	0	0	0	0	0	0	28
13:00 - 14:00	1	0	30	0	31	0	0	0	0	0	0	0	31
					В	REAK							
15:00 - 16:00	1	0	38	0	39	0	0	0	0	0	0	0	39
16:00 - 17:00	0	0	34	0	34	0	0	0	0	0	0	0	34
17:00 - 18:00	0	0	21	0	21	0	0	0	0	0	0	0	21
GRAND TOTAL	7	0	249	0	256	0	0	0	0	0	0	0	256



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

			Cars				Т	rucks			Bicycles					
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	4	9	0	0	13	1	0	0	0	1	0	0	0	0	0	0
07:15	4	17	0	0	21	0	1	0	0	1	0	0	0	0	0	0
07:30	8	18	0	0	26	2	2	0	0	4	0	0	0	0	0	0
07:45	7	30	0	0	37	3	3	0	0	6	0	0	0	0	0	0
08:00	6	26	0	0	32	0	3	0	0	3	0	0	0	0	0	0
08:15	10	30	0	0	40	2	5	0	0	7	0	0	0	0	0	0
08:30	2	23	0	0	25	1	5	0	0	6	0	0	0	0	0	0
08:45	4	17	0	0	21	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	45	170	0	0	215	9	21	0	0	30	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	-	1	Total	4	1	•	1	Total	Total Peds
11:00	5	19	0	0	24	1	1	0	0	2	0	0	0	0	0	0
11:15	4	15	0	0	19	0	3	0	0	3	0	0	0	0	0	0
11:30	8	15	0	0	23	2	4	0	0	6	0	0	0	0	0	0
11:45	9	21	0	0	30	3	3	0	0	6	0	0	0	0	0	0
12:00	7	17	0	0	24	0	2	0	0	2	0	0	0	0	0	0
12:15	9	22	0	0	31	2	0	0	0	2	0	0	0	0	0	0
12:30	6	28	0	0	34	2	1	0	0	3	0	0	0	0	0	0
12:45	12	24	0	0	36	0	1	0	0	1	0	0	0	0	0	0
13:00	8	14	0	0	22	0	4	0	0	4	0	0	0	0	0	0
13:15	9	19	0	0	28	0	1	0	0	1	0	0	0	0	0	0
13:30	7	22	0	0	29	1	0	0	0	1	0	0	0	0	0	0
13:45	7	23	0	0	30	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	91	239	0	0	330	11	20	0	0	31	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Thirty Rd

			Cars				1	rucks				В	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	9	19	0	0	28	0	2	0	0	2	0	0	0	0	0	0
15:15	11	42	0	0	53	0	2	0	0	2	0	0	0	0	0	0
15:30	12	37	0	0	49	2	2	0	0	4	0	0	0	0	0	0
15:45	20	39	0	0	59	0	2	0	0	2	0	0	0	0	0	0
16:00	15	37	0	0	52	3	2	0	0	5	0	0	0	0	0	0
16:15	10	37	0	0	47	0	1	0	0	1	0	0	0	0	0	0
16:30	13	45	0	0	58	0	3	0	0	3	0	0	0	0	0	0
16:45	10	51	0	0	61	0	2	0	0	2	0	0	0	0	0	0
17:00	6	41	0	0	47	1	2	0	0	3	0	0	0	0	0	0
17:15	13	42	0	0	55	0	1	0	0	1	0	0	0	0	0	0
17:30	6	43	0	0	49	1	3	0	0	4	0	0	0	0	0	0
17:45	9	26	0	0	35	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	134	459	0	0	593	7	23	0	0	30	0	0	0	0	0	0
GRAND TOTAL	270	868	0	0	1138	27	64	0	0	91	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

South Approach - Thirty Rd

			Cars				T	rucks				Ri	cycles			
Start Time	4	1	cars	a	Total	4	1	i ucns	Q	Total	4	1	cycles	a	Total	Total Peds
07:00	0	41	0	0	41	0	0	0	0	0	0	0	0	0	0	0
07:15	0	38	0	0	38	0	3	0	0	3	0	0	0	0	0	0
07:30	0	33	0	0	33	0	3	0	0	3	0	0	0	0	0	0
07:45	0	32	0	0	32	0	4	0	0	4	0	0	0	0	0	0
08:00	0	40	0	0	40	0	6	0	0	6	0	0	0	0	0	0
08:15	0	36	0	0	36	0	3	0	0	3	0	0	0	0	0	0
08:30	0	47	0	0	47	0	1	0	0	1	0	0	0	0	0	0
08:45	0	30	0	0	30	0	2	0	0	2	0	0	0	0	0	0
SUBTOTAL	0	297	0	0	297	0	22	0	0	22	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

South Approach - Thirty Rd

			Cars				Ti	rucks				Bio	cycles			
Start Time	4	1	•	1	Total	4	1	-	1	Total	4	1	•	Q	Total	Total Peds
11:00	0	29	0	0	29	0	2	0	0	2	0	0	0	0	0	0
11:15	0	29	0	0	29	0	2	0	0	2	0	0	0	0	0	0
11:30	0	32	0	0	32	0	0	0	0	0	0	0	0	0	0	0
11:45	0	15	0	0	15	0	4	1	0	5	0	0	0	0	0	0
12:00	0	20	0	0	20	0	2	0	0	2	0	0	0	0	0	0
12:15	0	19	0	0	19	0	3	0	0	3	0	0	0	0	0	0
12:30	0	19	0	0	19	0	3	0	0	3	0	0	0	0	0	0
12:45	0	24	0	0	24	0	2	0	0	2	0	0	0	0	0	0
13:00	0	22	1	0	23	0	1	0	0	1	0	0	0	0	0	0
13:15	0	16	0	0	16	0	3	0	0	3	0	0	0	0	0	0
13:30	0	19	0	0	19	0	1	0	0	1	0	0	0	0	0	0
13:45	0	34	0	0	34	0	4	0	0	4	0	0	0	0	0	0
SUBTOTAL	0	278	1	0	279	0	27	1	0	28	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

Count butc. Bet 01, 202

South Approach - Thirty Rd

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	0	45	0	0	45	0	0	0	0	0	0	0	0	0	0	0
15:15	0	35	0	0	35	0	1	0	0	1	0	0	0	0	0	0
15:30	0	31	0	0	31	0	2	0	0	2	0	0	0	0	0	0
15:45	0	36	0	0	36	0	2	0	0	2	0	0	0	0	0	0
16:00	0	29	0	0	29	0	0	0	0	0	0	0	0	0	0	0
16:15	0	49	0	0	49	0	0	0	0	0	0	0	0	0	0	0
16:30	0	48	0	0	48	0	3	0	0	3	0	0	0	0	0	0
16:45	0	40	0	0	40	0	0	0	0	0	0	0	0	0	0	0
17:00	0	33	0	0	33	0	2	0	0	2	0	0	0	0	0	0
17:15	0	33	0	0	33	0	1	0	0	1	0	0	0	0	0	0
17:30	0	20	0	0	20	0	1	0	0	1	0	0	0	0	0	0
17:45	0	26	0	0	26	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	425	0	0	425	0	12	0	0	12	0	0	0	0	0	0
GRAND TOTAL	0	1000	1	0	1001	0	61	1	0	62	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Clayson Rd

			Cars				Ti	rucks				Bi	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	0
07:15	0	0	11	0	11	1	0	4	0	5	0	0	0	0	0	0
07:30	0	0	16	0	16	0	0	1	0	1	0	0	0	0	0	0
07:45	0	0	7	0	7	0	0	0	0	0	0	0	0	0	0	0
08:00	0	0	10	0	10	0	0	1	0	1	0	0	0	0	0	0
08:15	0	0	6	0	6	0	0	1	0	1	0	0	0	0	0	0
08:30	0	0	8	0	8	0	0	2	0	2	0	0	0	0	0	0
08:45	0	0	5	0	5	1	0	2	0	3	0	0	0	0	0	0
SUBTOTAL	0	0	69	0	69	2	0	11	0	13	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001

Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Clayson Rd

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	0	0	3	0	3	0	0	1	0	1	0	0	0	0	0	0
11:15	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0	0
11:30	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0	0
11:45	0	0	7	0	7	0	0	0	0	0	0	0	0	0	0	0
12:00	1	0	6	0	7	0	0	2	0	2	0	0	0	0	0	0
12:15	0	0	4	0	4	0	0	1	0	1	0	0	0	0	0	0
12:30	0	0	8	0	8	2	0	1	0	3	0	0	0	0	0	0
12:45	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0
13:00	1	0	3	0	4	0	0	0	0	0	0	0	0	0	0	0
13:15	0	0	5	0	5	0	0	1	0	1	0	0	0	0	0	0
13:30	0	0	11	0	11	0	0	0	0	0	0	0	0	0	0	0
13:45	0	0	9	0	9	0	0	1	0	1	0	0	0	0	0	0
SUBTOTAL	2	0	69	0	71	2	0	7	0	9	0	0	0	0	0	0



Intersection: Thirty Rd & Clayson Rd

Site Code: 2241000001 Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Clayson Rd

			Cars				T	rucks				Bi	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	0	0	12	0	12	0	0	0	0	0	0	0	0	0	0	0
15:15	0	0	5	0	5	1	0	0	0	1	0	0	0	0	0	0
15:30	0	0	11	0	11	0	0	0	0	0	0	0	0	0	0	0
15:45	0	0	8	0	8	0	0	2	0	2	0	0	0	0	0	0
16:00	0	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0
16:15	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	0
16:30	0	0	16	0	16	0	0	2	0	2	0	0	0	0	0	0
16:45	0	0	5	0	5	0	0	1	0	1	0	0	0	0	0	0
17:00	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	0
17:15	0	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0
17:30	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0	0
17:45	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	0	88	0	88	1	0	5	0	6	0	0	0	0	0	0
GRAND TOTAL	2	0	226	0	228	5	0	23	0	28	0	0	0	0	0	0



Peak Hour Diagram

Specified Period

One Hour Peak

From: 07:00:00 To: 09:00:00 From: 07:45:00 To: 08:45:00

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	134	186	320
	22	18	40
<i>₫</i>	0	0	0
	156	204	360

Thirty Rd

	1	L	Ú
Totals	12!	5 31	0
	109	9 25	0
	1	6 6	0
<i>₫</i>		0 0	0

East Approach

	Out	In	Total
	31	25	56
	4	6	10
ॐ	0	0	0
,	35	31	66

Peds: 0







Clayson Rd

	Totals			<i>₹</i> 6
C	0	0	0	0
£	35	31	4	0
F	0	0	0	0

Peds: 0

	1		J.
Totals	169	0	0
	155	0	0
	14	0	0
<i>₫</i>	0	0	0

Thirty Rd

South Approach

	Out	In	Total
	155	109	264
	14	16	30
<i>₫</i>	0	0	0
	169	125	294



🚨 - Trucks

- Bicycles

Comments



Peak Hour Summary

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

 Period:
 07:00 - 09:00

Peak Hour Data (07:45 - 08:45)

		ı	North A Thir	pproac ty Rd	h			S	outh <i>A</i> Thir	ipproac ty Rd	h				East A _l Clays	pproach son Rd	1				West A	Approacl	h		Total Vehicl
Start Time	4	1		J	Peds	Total	4	1		J	Peds	Total	•	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
07:45	10	33		0	0	43		36	0	0	0	36	0		7	0	0	7					0		86
08:00	6	29		0	0	35		46	0	0	0	46	0		11	0	0	11					0		92
08:15	12	35		0	0	47		39	0	0	0	39	0		7	0	0	7					0		93
08:30	3	28		0	0	31		48	0	0	0	48	0		10	0	0	10					0		89
Grand Total	31	125		0	0	156		169	0	0	0	169	0		35	0	0	35					0	0	360
Approach %	19.9	80.1		0		-		100	0	0		-	0		100	0		-						-	
Totals %	8.6	34.7		0		43.3		46.9	0	0	,	46.9	0	,	9.7	0	,	9.7						0	
PHF	0.65	0.89		0		0.83		0.88	0	0		0.88	0		0.8	0		0.8						0	0.97
Cars	25	109		0		134		155	0	0		155	0		31	0		31						0	320
% Cars	80.6	87.2		0		85.9		91.7	0	0		91.7	0		88.6	0		88.6						0	88.9
Trucks	6	16		0		22		14	0	0		14	0		4	0		4						0	40
% Trucks	19.4	12.8		0		14.1		8.3	0	0		8.3	0		11.4	0		11.4						0	11.1
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 11:00:00 To: 14:00:00 From: 12:00:00 To: 13:00:00

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	125	103	228
	8	14	22
<i>₹</i>	0	0	0
	133	117	250

Thirty Rd

	1	14	Ú
Totals	95	38	0
	91	34	0
	4	4	0
₫ %	0	0	0

East Approach

	Out	In	Total
	22	34	56
	6	4	10
ॐ	0	0	0
,	28	38	66

Peds: 0







Clayson Rd

	Totals			<i>₫</i> 6
C	0	0	0	0
£	25	21	4	0
F	3	1	2	0

Peds: 0

	1		J.
Totals	92	0	0
	82	0	0
	10	0	0
<i>₫</i>	0	0	0

Thirty Rd

South Approach

	Out	In	Total
	82	92	174
	10	6	16
<i>₫</i>	0	0	0
	92	98	190







Comments



Peak Hour Summary

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

Period: 11:00 - 14:00

Peak Hour Data (12:00 - 13:00)

		N	North A Thirt	pproac ty Rd	h			S	outh A Thir	ipproac ty Rd	h				East Ap Clays	oproach on Rd	1				West A	Approac	h		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
12:00	7	19		0	0	26		22	0	0	0	22	1		8	0	0	9					0		57
12:15	11	22		0	0	33		22	0	0	0	22	0		5	0	0	5					0		60
12:30	8	29		0	0	37		22	0	0	0	22	2		9	0	0	11					0		70
12:45	12	25		0	0	37		26	0	0	0	26	0		3	0	0	3					0		66
Grand Total	38	95		0	0	133		92	0	0	0	92	3		25	0	0	28					0	0	253
Approach %	28.6	71.4		0		-		100	0	0		-	10.7		89.3	0		-						-	
Totals %	15	37.5	,	0		52.6		36.4	0	0		36.4	1.2		9.9	0	,	11.1					,	0	
PHF	0.79	0.82		0		0.9		0.88	0	0		0.88	0.38		0.69	0		0.64						0	0.9
Cars	34	91		0		125		82	0	0		82	1		21	0		22						0	229
% Cars	89.5	95.8		0		94		89.1	0	0		89.1	33.3		84	0		78.6						0	90.5
Trucks	4	4		0		8		10	0	0		10	2		4	0		6						0	24
% Trucks	10.5	4.2		0		6		10.9	0	0		10.9	66.7		16	0		21.4						0	9.5
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 15:00:00 To: 18:00:00 From: 16:15:00 To: 17:15:00

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

** Unsignalized Intersection **

Major Road: Thirty Rd runs N/S

North Approach

	Out	In	Total
	213	203	416
	9	8	17
<i>₫</i>	0	0	0
	222	211	433

Thirty Rd

	1	L	Ú
Totals	182	40	0
	174	39	0
	8	1	0
<i>₫</i>	0	0	0

East Approach

	Out	In	Total
	33	39	72
	3	1	4
₩	0	0	0
	36	40	76

Peds: 0





Clayson Rd

	Totals			<i>₫</i> 6
C	0	0	0	0
Ł	36	33	3	0
F	0	0	0	0

Peds: 0

	1		J
Totals	175	0	0
	170	0	0
₽	5	0	0
<i>₹</i>	0	0	0

Thirty Rd

South Approach

	Out	In	Total
	170	174	344
	5	8	13
<i>₫</i>	0	0	0
	175	182	357



🚨 - Trucks

♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Thirty Rd & Clayson Rd

 Site Code:
 2241000001

 Count Date:
 Dec 01, 2022

 Period:
 15:00 - 18:00

Peak Hour Data (16:15 - 17:15)

		ı	North A Thir	pproac ty Rd	h			S	outh <i>A</i> Thir	ipproac ty Rd	:h				East A _l Clays	pproach son Rd	1		West Approach					Total Vehicl	
Start Time	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	•	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
16:15	10	38		0	0	48		49	0	0	0	49	0		6	0	0	6					0		103
16:30	13	48		0	0	61		51	0	0	0	51	0		18	0	0	18					0		130
16:45	10	53		0	0	63		40	0	0	0	40	0		6	0	0	6					0		109
17:00	7	43		0	0	50		35	0	0	0	35	0		6	0	0	6					0		91
Grand Total	40	182		0	0	222		175	0	0	0	175	0		36	0	0	36					0	0	433
Approach %	18	82		0		-		100	0	0		-	0		100	0		-						-	
Totals %	9.2	42		0		51.3		40.4	0	0	,	40.4	0	,	8.3	0		8.3			,	_		0	
PHF	0.77	0.86		0		0.88		0.86	0	0		0.86	0		0.5	0		0.5						0	0.83
Cars	39	174		0		213		170	0	0		170	0		33	0		33						0	416
% Cars	97.5	95.6		0		95.9		97.1	0	0		97.1	0		91.7	0		91.7						0	96.1
Trucks	1	8		0		9		5	0	0		5	0		3	0		3						0	17
% Trucks	2.5	4.4		0		4.1		2.9	0	0		2.9	0		8.3	0		8.3						0	3.9
Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
% Bicycles	0	0		0		0		0	0	0		0	0		0	0		0						0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0		



Project #22-410 - RV Anderson

Intersection Count Report

Intersection: Young St & Clayson Rd

Municipality: Smithville

Count Date: Thursday, Dec 01, 2022

Site Code: 2241000003

Count Categories: Cars, Trucks, Bicycles, Pedestrians

Count Period: 07:00-09:00, 11:00-14:00, 15:00-18:00

Weather: Clear

Comments:

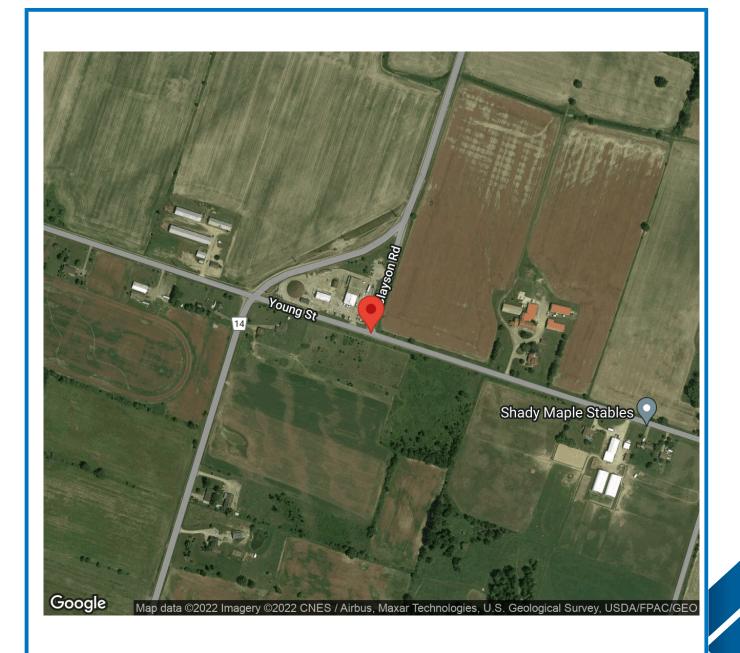


Traffic Count Map

Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022





Traffic Count Summary

Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

Clayson Rd - Traffic Summary

		North	Appr	oach T	otals		South Approach Totals						
		Include	s Cars, 1	Γrucks, Bi	cycles								
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	30	0	0	0	30	0	0	0	0	0	0	0	30
08:00 - 09:00	25	0	0	0	25	0	0	0	0	0	0	0	25
BREAK													
11:00 - 12:00	31	0	0	0	31	0	0	0	0	0	0	0	31
12:00 - 13:00	38	0	0	0	38	0	0	0	0	0	0	0	38
13:00 - 14:00	31	0	1	0	32	0	0	0	0	0	0	0	32
					BI	REAK							
15:00 - 16:00	54	0	3	0	57	0	0	0	0	0	0	0	57
16:00 - 17:00	49	0	1	0	50	0	0	0	0	0	0	0	50
17:00 - 18:00	36	0	0	0	36	0	0	0	0	0	0	0	36
GRAND TOTAL	294	0	5	0	299	0	0	0	0	0	0	0	299



Traffic Count Summary

Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

Young St - Traffic Summary

		East A	Appro	ach To	tals								
		Include	s Cars, 1	Γrucks, Bi	cycles								
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	0	31	45	0	76	0	0	29	0	0	29	0	105
08:00 - 09:00	0	20	34	0	54	0	2	28	0	0	30	0	84
BREAK													
11:00 - 12:00	0	14	23	0	37	0	1	19	0	0	20	0	57
12:00 - 13:00	0	20	25	0	45	0	0	25	0	0	25	0	70
13:00 - 14:00	0	26	30	0	56	0	0	31	0	0	31	0	87
					В	REAK .							
15:00 - 16:00	0	32	35	0	67	0	1	20	0	0	21	0	88
16:00 - 17:00	0	30	35	0	65	0	0	50	0	0	50	0	115
17:00 - 18:00	0	20	20	0	40	0	0	39	0	0	39	0	79
GRAND TOTAL	0	193	247	0	440	0	4	241	0	0	245	0	685



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Clayson Rd

			Cars				T	rucks				Bi	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	5	0	0	0	5	1	0	0	0	1	0	0	0	0	0	0
07:15	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0
07:30	8	0	0	0	8	2	0	0	0	2	0	0	0	0	0	0
07:45	7	0	0	0	7	3	0	0	0	3	0	0	0	0	0	0
08:00	6	0	0	0	6	0	0	0	0	0	0	0	0	0	0	0
08:15	10	0	0	0	10	2	0	0	0	2	0	0	0	0	0	0
08:30	2	0	0	0	2	1	0	0	0	1	0	0	0	0	0	0
08:45	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	46	0	0	0	46	9	0	0	0	9	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Clayson Rd

			Cars				T	rucks				Bi	cycles			
Start Time	4	1	•	Q	Total	4	1	•	Q.	Total	4	1	•	1	Total	Total Peds
11:00	5	0	0	0	5	1	0	0	0	1	0	0	0	0	0	0
11:15	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0
11:30	8	0	0	0	8	1	0	0	0	1	0	0	0	0	0	0
11:45	9	0	0	0	9	3	0	0	0	3	0	0	0	0	0	0
12:00	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0
12:15	9	0	0	0	9	2	0	0	0	2	0	0	0	0	0	0
12:30	6	0	0	0	6	2	0	0	0	2	0	0	0	0	0	0
12:45	12	0	0	0	12	0	0	0	0	0	0	0	0	0	0	0
13:00	7	0	1	0	8	0	0	0	0	0	0	0	0	0	0	0
13:15	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0
13:30	7	0	0	0	7	1	0	0	0	1	0	0	0	0	0	0
13:45	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	90	0	1	0	91	10	0	0	0	10	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

North Approach - Clayson Rd

			Cars				T	rucks				Bi	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	10	0	3	0	13	0	0	0	0	0	0	0	0	0	0	0
15:15	10	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0
15:30	12	0	0	0	12	2	0	0	0	2	0	0	0	0	0	0
15:45	20	0	0	0	20	0	0	0	0	0	0	0	0	0	0	0
16:00	15	0	0	0	15	3	0	0	0	3	0	0	0	0	0	0
16:15	10	0	0	0	10	0	0	0	0	0	0	0	0	0	0	0
16:30	13	0	0	0	13	0	0	0	0	0	0	0	0	0	0	0
16:45	8	0	1	0	9	0	0	0	0	0	0	0	0	0	0	0
17:00	6	0	0	0	6	1	0	0	0	1	0	0	0	0	0	0
17:15	13	0	0	0	13	0	0	0	0	0	0	0	0	0	0	0
17:30	7	0	0	0	7	0	0	0	0	0	0	0	0	0	0	0
17:45	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	133	0	4	0	137	6	0	0	0	6	0	0	0	0	0	0
GRAND TOTAL	269	0	5	0	274	25	0	0	0	25	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003

Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				T	rucks				Bi	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	3	6	0	9	0	0	1	0	1	0	0	0	0	0	0
07:15	0	7	11	0	18	0	0	3	0	3	0	0	0	0	0	0
07:30	0	8	16	0	24	0	4	1	0	5	0	0	0	0	0	0
07:45	0	7	7	0	14	0	2	0	0	2	0	0	0	0	0	0
08:00	0	0	10	0	10	0	1	0	0	1	0	0	0	0	0	0
08:15	0	5	5	0	10	0	1	0	0	1	0	0	0	0	0	0
08:30	0	7	8	0	15	0	0	3	0	3	0	0	0	0	0	0
08:45	0	6	5	0	11	0	0	3	0	3	0	0	0	0	0	0
SUBTOTAL	0	43	68	0	111	0	8	11	0	19	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003

Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	0	5	3	0	8	0	0	1	0	1	0	0	0	0	0	0
11:15	0	1	6	0	7	0	0	0	0	0	0	0	0	0	0	0
11:30	0	4	4	0	8	0	1	2	0	3	0	0	0	0	0	0
11:45	0	3	7	0	10	0	0	0	0	0	0	0	0	0	0	0
12:00	0	4	8	0	12	0	1	1	0	2	0	0	0	0	0	0
12:15	0	6	4	0	10	0	1	1	0	2	0	0	0	0	0	0
12:30	0	2	7	0	9	0	2	1	0	3	0	0	0	0	0	0
12:45	0	2	3	0	5	0	2	0	0	2	0	0	0	0	0	0
13:00	0	4	3	0	7	0	3	0	0	3	0	0	0	0	0	0
13:15	0	3	5	0	8	0	3	1	0	4	0	0	0	0	0	0
13:30	0	4	11	0	15	0	2	0	0	2	0	0	1	0	1	0
13:45	0	5	8	0	13	0	2	1	0	3	0	0	0	0	0	0
SUBTOTAL	0	43	69	0	112	0	17	8	0	25	0	0	1	0	1	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

East Approach - Young St

			Cars				Tı	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1	•	Q.	Total	4	1	P	1	Total	Total Peds
15:00	0	8	10	0	18	0	0	0	0	0	0	0	0	0	0	0
15:15	0	4	4	0	8	0	1	0	0	1	0	0	0	0	0	0
15:30	0	7	11	0	18	0	4	0	0	4	0	0	0	0	0	0
15:45	0	7	8	0	15	0	1	2	0	3	0	0	0	0	0	0
16:00	0	7	4	0	11	0	0	0	0	0	0	0	0	0	0	0
16:15	0	6	6	0	12	0	1	0	0	1	0	0	0	0	0	0
16:30	0	6	16	0	22	0	0	2	0	2	0	0	0	0	0	0
16:45	0	10	6	0	16	0	0	1	0	1	0	0	0	0	0	0
17:00	0	5	5	0	10	0	0	0	0	0	0	0	0	0	0	0
17:15	0	10	4	0	14	0	0	0	0	0	0	0	0	0	0	0
17:30	0	1	6	0	7	0	0	0	0	0	0	0	0	0	0	0
17:45	0	4	5	0	9	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	75	85	0	160	0	7	5	0	12	0	0	0	0	0	0
GRAND TOTAL	0	161	222	0	383	0	32	24	0	56	0	0	1	0	1	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003

Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				T	rucks				В	icycles			
Start Time	4	1	•	Q	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	0	6	0	0	6	0	1	0	0	1	0	0	0	0	0	0
07:15	0	5	0	0	5	0	2	0	0	2	0	0	0	0	0	0
07:30	0	6	0	0	6	0	2	0	0	2	0	0	0	0	0	0
07:45	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	0
08:00	0	5	0	0	5	0	1	0	0	1	0	0	0	0	0	0
08:15	1	3	0	0	4	0	1	0	0	1	0	0	0	0	0	0
08:30	0	8	0	0	8	0	1	0	0	1	0	0	0	0	0	0
08:45	0	9	0	0	9	1	0	0	0	1	0	0	0	0	0	0
SUBTOTAL	1	49	0	0	50	1	8	0	0	9	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				1	rucks				Bi	cycles			
Start Time	4	1	•	1	Total	4	1		1	Total	4	1	•	1	Total	Total Peds
11:00	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
11:15	0	4	0	0	4	0	1	0	0	1	0	0	0	0	0	0
11:30	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	0
11:45	0	2	0	0	2	1	1	0	0	2	0	0	0	0	0	0
12:00	0	3	0	0	3	0	1	0	0	1	0	0	0	0	0	0
12:15	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0
12:30	0	6	0	0	6	0	1	0	0	1	0	0	0	0	0	0
12:45	0	9	0	0	9	0	2	0	0	2	0	0	0	0	0	0
13:00	0	4	0	0	4	0	1	0	0	1	0	0	0	0	0	0
13:15	0	8	0	0	8	0	1	0	0	1	0	0	0	0	0	0
13:30	0	4	0	0	4	0	3	0	0	3	0	0	0	0	0	0
13:45	0	9	0	0	9	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	0	61	0	0	61	1	14	0	0	15	0	0	0	0	0	0



Intersection: Young St & Clayson Rd

Site Code: 2241000003 Municipality: Smithville

Count Date: Dec 01, 2022

West Approach - Young St

			Cars				1	rucks				В	icycles			
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	0	4	0	0	4	0	1	0	0	1	0	0	0	0	0	0
15:15	0	2	0	0	2	1	1	0	0	2	0	0	0	0	0	0
15:30	0	5	0	0	5	0	3	0	0	3	0	0	0	0	0	0
15:45	0	3	0	0	3	0	1	0	0	1	0	0	0	0	0	0
16:00	0	11	0	0	11	0	0	0	0	0	0	0	0	0	0	0
16:15	0	10	0	0	10	0	2	0	0	2	0	0	0	0	0	0
16:30	0	13	0	0	13	0	0	0	0	0	0	0	0	0	0	0
16:45	0	14	0	0	14	0	0	0	0	0	0	0	0	0	0	0
17:00	0	14	0	0	14	0	0	0	0	0	0	0	0	0	0	0
17:15	0	6	0	0	6	0	0	0	0	0	0	0	0	0	0	0
17:30	0	8	0	0	8	0	0	0	0	0	0	0	0	0	0	0
17:45	0	11	0	0	11	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	101	0	0	101	1	8	0	0	9	0	0	0	0	0	0
GRAND TOTAL	1	211	0	0	212	3	30	0	0	33	0	0	0	0	0	0



Peak Hour Diagram

Specified Period

One Hour Peak

From: To: 07:00:00 09:00:00

From: 07:00:00 To: 08:00:00

Intersection:

Young St & Clayson Rd

Site Code: Count Date: 2241000003 Dec 01, 2022 Weather conditions:

Clear

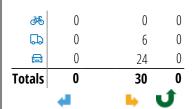
** Unsignalized Intersection **

Major Road: Young St runs E/W

North Approach

	Out	In	Total
	24	40	64
	6	5	11
<i>₫</i>	0	0	0
	30	45	75

Clayson Rd



East Approach

	Out	In	Total
	65	48	113
	11	11	22
ॐ	0	0	0
	76	59	135

Young St

	Totals			<i>₹</i>	
7	0	0	0	0	
4	0	0	0	0	
→	29	24	5	0	

Peds: 0

Peds: 0



Peds: 0

Young St

	Totals			₫ %
C	0	0	0	0
£	45	40	5	0
—	31	25	6	0

West Approach

	Out	In	Total
	24	25	49
	5	6	11
<i>₹</i>	0	0	0
	29	31	60

📾 - Cars

🚨 - Trucks

Bicycles

Comments



Peak Hour Summary

Intersection: Young St & Clayson Rd

 Site Code:
 2241000003

 Count Date:
 Dec 01, 2022

 Period:
 07:00 - 09:00

Peak Hour Data (07:00 - 08:00)

		ı	North <i>A</i> Clays	Approac son Rd	h				South /	Approac	h				East Ap You	pproacl ng St	1			,	West A You	pproach ng St	1		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	P	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	es
07:00	6		0	0	0	6					0			3	7	0	0	10	0	7		0	0	7	23
07:15	4		0	0	0	4					0			7	14	0	0	21	0	7		0	0	7	32
07:30	10		0	0	0	10					0			12	17	0	0	29	0	8		0	0	8	47
07:45	10		0	0	0	10					0			9	7	0	0	16	0	7		0	0	7	33
Grand Total	30		0	0	0	30					0	0		31	45	0	0	76	0	29		0	0	29	135
Approach %	100		0	0		-						-		40.8	59.2	0		-	0	100		0		-	
Totals %	22.2		0	0		22.2				,	,	0		23	33.3	0	,	56.3	0	21.5		0	,	21.5	
PHF	0.75		0	0		0.75						0		0.65	0.66	0		0.66	0	0.91		0		0.91	0.72
Cars	24		0	0		24						0		25	40	0		65	0	24		0		24	113
% Cars	80		0	0		80						0		80.6	88.9	0		85.5	0	82.8		0		82.8	83.7
Trucks	6		0	0		6						0		6	5	0		11	0	5		0		5	22
% Trucks	20		0	0		20						0		19.4	11.1	0		14.5	0	17.2		0		17.2	16.3
Bicycles	0		0	0		0						0		0	0	0		0	0	0		0		0	0
% Bicycles	0		0	0		0						0		0	0	0		0	0	0		0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

From: 11:00:00 To: 14:00:00 From: 13:00:00 To: 14:00:00

Intersection: Young St & Clayson Rd

 Site Code:
 2241000003

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

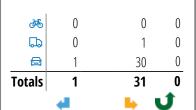
** Unsignalized Intersection **

Major Road: Young St runs E/W

North Approach

	Out	In	Total
	31	27	58
	1	2	3
₫	0	1	1
,	32	30	62

Clayson Rd



East Approach

	Out	In	Total
	43	55	98
	12	7	19
₩	1	0	1
	56	62	118

Young St

	Totals			<i>₫</i>
7	0	0	0	0
4	0	0	0	0
\Rightarrow	31	25	6	0

Peds: 0

Peds: 0



Peds: 0

Peds: 0

Young St

	Totals			<i>₹</i>
C	0	0	0	0
Ł	30 26	27	2	1
(26	16	10	0

West Approach

	Out	In	Total
	25	17	42
	6	10	16
ॐ	0	0	0
,	31	27	58

📾 - Cars

🞝 - Trucks

♣ - Bicycles

Comments



Peak Hour Summary

Intersection: Young St & Clayson Rd

 Site Code:
 2241000003

 Count Date:
 Dec 01, 2022

 Period:
 11:00 - 14:00

Peak Hour Data (13:00 - 14:00)

		ı	North A Clays	pproac on Rd	h				South /	Approac	:h				East A _l You	oproach ng St	1				West A You	pproacl ng St	h		Total Vehicl
Start Time	•	1	P	J	Peds	Total	4	1	•	J	Peds	Total	4	1	•	J	Peds	Total	4	1		J	Peds	Total	es
13:00	7		1	0	0	8					0			7	3	0	0	10	0	5		0	0	5	23
13:15	9		0	0	0	9					0			6	6	0	0	12	0	9		0	0	9	30
13:30	8		0	0	0	8					0			6	12	0	0	18	0	7		0	0	7	33
13:45	7		0	0	0	7					0			7	9	0	0	16	0	10		0	0	10	33
Grand Total	31		1	0	0	32					0	0		26	30	0	0	56	0	31		0	0	31	119
Approach %	96.9		3.1	0		-						-		46.4	53.6	0		-	0	100		0		-	
Totals %	26.1		0.8	0		26.9					,	0		21.8	25.2	0	,	47.1	0	26.1		0		26.1	
PHF	0.86		0.25	0		0.89						0		0.93	0.63	0		0.78	0	0.78		0		0.78	0.9
Cars	30		1	0		31						0		16	27	0		43	0	25		0		25	99
% Cars	96.8		100	0		96.9						0		61.5	90	0		76.8	0	80.6		0		80.6	83.2
Trucks	1		0	0		1						0		10	2	0		12	0	6		0		6	19
% Trucks	3.2		0	0		3.1						0		38.5	6.7	0		21.4	0	19.4		0		19.4	16
Bicycles	0		0	0		0						0		0	1	0		1	0	0		0		0	1
% Bicycles	0		0	0		0						0		0	3.3	0		1.8	0	0		0		0	0.8
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	



Peak Hour Diagram

Specified Period

One Hour Peak

To:

From: 15:00:00 To: 18:00:00 From: 15:45:00

16:45:00

Intersection: Young St & Clayson Rd

 Site Code:
 2241000003

 Count Date:
 Dec 01, 2022

Weather conditions:

Clear

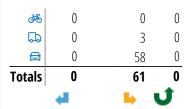
** Unsignalized Intersection **

Major Road: Young St runs E/W

North Approach

	Out	In	Total
	58	34	92
	3	4	7
<i>₹</i>	0	0	0
	61	38	99

Clayson Rd



East Approach

	Out	In	Total
	60	95	155
	6	6	12
₫	0	0	0
	66	101	167

Young St

	Totals			<i>₫</i>
7	0	0	0	0
4	0	0	0	0
\Rightarrow	40	37	3	0

Peds: 0

Peds: 0



Peds: 0

Peds: 0

Young St

	Totals			<i>₫</i>
C	0	0	0	0
£	38	34	4	0
(=	38 28	26	2	0

West Approach

	Out	In	Total
	37	26	63
	3	2	5
<i>₹</i>	0	0	0
	40	28	68

📾 - Cars

🚨 - Trucks

Bicycles

Comments



Peak Hour Summary

Intersection: Young St & Clayson Rd

 Site Code:
 2241000003

 Count Date:
 Dec 01, 2022

Period: 15:00 - 18:00

Peak Hour Data (15:45 - 16:45)

		ı	North <i>F</i> Clays	Approac son Rd	:h				South /	Approac	:h				East Ap You	proacl ng St	1			Ī	West A You	pproach ng St	1		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1	P	J	Peds	Total	4	1	•	J	Peds	Total	4	1	P	J	Peds	Total	es
15:45	20		0	0	0	20					0			8	10	0	0	18	0	4		0	0	4	42
16:00	18		0	0	0	18					0			7	4	0	0	11	0	11		0	0	11	40
16:15	10		0	0	0	10					0			7	6	0	0	13	0	12		0	0	12	35
16:30	13		0	0	0	13					0			6	18	0	0	24	0	13		0	0	13	50
Grand Total	61		0	0	0	61					0	0		28	38	0	0	66	0	40		0	0	40	167
Approach %	100		0	0		-						-		42.4	57.6	0		-	0	100		0		-	
Totals %	36.5		0	0	,	36.5				,	,	0		16.8	22.8	0	,	39.5	0	24		0	,	24	
PHF	0.76		0	0		0.76						0		0.88	0.53	0		0.69	0	0.77		0		0.77	0.84
Cars	58		0	0		58						0		26	34	0		60	0	37		0		37	155
% Cars	95.1		0	0		95.1						0		92.9	89.5	0		90.9	0	92.5		0		92.5	92.8
Trucks	3		0	0		3						0		2	4	0		6	0	3		0		3	12
% Trucks	4.9		0	0		4.9						0		7.1	10.5	0		9.1	0	7.5		0		7.5	7.2
Bicycles	0		0	0		0						0		0	0	0		0	0	0		0		0	0
% Bicycles	0		0	0		0						0		0	0	0		0	0	0		0		0	0
Peds					0	-					0	-					0	-					0	-	0
% Peds					0	-					0	-					0	-					0	-	

APPENDIX 2

Historical Collision Data





From: January 1, 2017 To: August 22, 2022

Location Clayson Road @ Thirty Road Municipality....... WEST LINCOLN

Traffic Control.... Unknown Total Collisions.... 1

Collision ID Date/Day/Time Vehicle Manoeuver Vehicle type First Event **Driver Action** No. Ped Environment Impact Type Classification Direction Surface Cond'n 219800 2021-Feb-02, Tue,22:33 Drifting Snow SMV other P.D. only North Loose snow Going ahead Automobile, Concrete guide Driving properly station wagon

Comments: Loose snow



Location Clayson Road @ Young Street

Traffic Control.... Unknown

Collision Details Report

From: January 1, 2017 To: August 22, 2022

Municipality...... WEST LINCOLN

Total Collisions.... 2

Collision ID	Date/Day/Time	Environment	Impact Type	Classification	Direction	Surface Cond'n	Vehicle Manoeuver	Vehicle type	First Event	Driver Action	No. Ped
17118068	2017-Dec-08, Fri,09:55	Snow	Other	P.D. only	South	Wet	Stopped	Automobile, station wagon	Other motor vehicle	Driving properly	
Comments	: d2 charged				South	Wet	Reversing	Truck - tractor	Other motor vehicle	Failed to yield right-of- way	-
17122869	2017-Dec-23, Sat,00:19	Freezing Rain	SMV other	P.D. only	South	Ice	Stopped	Automobile, station wagon	Skidding/sliding	Driving properly	
Comments						Ice		otation wagon			



From: January 1, 2017 To: August 22, 2022

Location Thirty Road @ Young Street Municipality....... WEST LINCOLN

Traffic Control.... Stop sign Total Collisions.... 14

Collision ID	Date/Day/Time	Environment	Impact Type	Classification	Direction	Surface Cond'n	Vehicle Manoeuver	Vehicle type	First Event	Driver Action	No. Ped
	•							7.			110.1 00
1772233	2017-Aug-03, Thu,17:23	Clear	Angle	P.D. only	East	Dry	Going ahead	Pick-up truck	Other motor vehicle	Disobeyed traffic control	
Comments	: d1 charged				South	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	
17107379	2017-Nov-06, Mon,07:44	Clear	Angle	P.D. only	West	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-c	f-
Comments	: d1 charged				South	Dry	Going ahead	Truck - closed	Other motor vehicle	Driving properly	
1820500	2018-Mar-09, Fri,16:11	Clear	Angle	P.D. only	West	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-c	f-
Comments	: d1 charged				South	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	
1881058	2018-Aug-18, Sat,13:30	Clear	Other	Non-fatal injur	y East	Dry	Going ahead	Passenger van	Skidding/sliding	Failed to yield right-o	f-
Comments	: d1 charged				South	Dry	Going ahead	Motorcycle	Ran off road	Driving properly	
1940325	2019-May-07, Tue,07:14	Rain	Angle	P.D. only	West	Wet	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-o	f-
Comments	: d1 charged				North	Wet	Going ahead	Pick-up truck	Other motor vehicle	Driving properly	
1961432	2019-Jul-01, Mon,13:56	Clear	Angle	P.D. only	South	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	
Comments	: d2 charged				West	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-o	f-
19116838	2019-Nov-16, Sat,17:59	Clear	Angle	P.D. only	East	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-c	f-
Comments	:				South	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	
1609	2020-Jan-06, Mon,08:30	Clear	Angle	P.D. only	West	Wet	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-o	f-
Comments	:				North	Wet	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	
20111991	2020-Nov-16, Mon,16:03	Clear	Angle	P.D. only	West	Dry	Going ahead	Passenger van	Other motor vehicle	Other	
Comments	:				South	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly	

2128201	2021-Mar-30, Tue,17:10 Clea	r Angle	Non-fatal injur	y East	Dry	Going ahead	Pick-up truck	Other motor vehicle	Failed to yield right-of- way
Comments	s:			North	Dry	Going ahead	Passenger van	Other motor vehicle	Driving properly
2151898	2021-May-28, Fri,13:25 Rain	Angle	P.D. only	West	Wet	Going ahead	Automobile, station wagon	Other motor vehicle	Failed to yield right-of- way
Comments	s:			South	Wet	Going ahead	Pick-up truck	Other motor vehicle	Driving properly
2169451	2021-Jul-03, Sat,10:36 Clea	r Angle	Non-fatal injur	y South	Dry	Going ahead	Pick-up truck	Other motor vehicle	Disobeyed traffic control
Comments	s:			West	Dry	Going ahead	Pick-up truck	Other motor vehicle	Driving properly
2188642	2021-Aug-11, Wed,11:20 Clea	r Turning movement	P.D. only	West	Dry	Turning left	Automobile, station wagon	Other motor vehicle	Driving properly
Comments	s:			East	Dry	Going ahead	Automobile, station wagon	Other motor vehicle	Driving properly
2270903	2022-Jun-29, Wed,19:01 Clea	r SMV other	P.D. only	North	Dry	Going ahead	Automobile, station wagon	Ran off road	Other
Comments	s:								



From: January 1, 2017 To: August 22, 2022

Location Thirty Road btwn Spring Creek Road & Station Street & Young Street

Municipality...... WEST LINCOLN

Traffic Control.... Unknown

Total Collisions.... 5

manne oc	ontrol Onknown							Total O	JIII310113 J		
Collision ID	Date/Day/Time	Environment	Impact Type	Classification	Direction	Surface Cond'n	Vehicle Manoeuver	Vehicle type	First Event	Driver Action	No. Ped
171778	2017-Jan-07, Sat,02:19	Clear	SMV other	P.D. only	North	Dry	Pulling onto shoulder or toward curb	Automobile, station wagon	Ran off road	Lost control	
Comments	: d1 charged										
1837759	2018-May-02, Wed,21:15	5 Clear	SMV other	P.D. only	North	Dry	Going ahead	Automobile, station wagon	Animal - wild	Driving properly	
Comments	: deer										
20117303	2020-Dec-03, Thu,00:07	Clear	SMV other	P.D. only	East	Dry	Going ahead	Pick-up truck	Ran off road	Lost control	
Comments	:										
2229857	2022-Mar-24, Thu,13:08	Clear	SMV other	P.D. only	South	Dry	Going ahead	Automobile, station wagon	Ran off road	Other	
Comments	:							· ·			
2254106	2022-May-22, Sun,18:34	Clear	SMV other	P.D. only	South	Dry	Going ahead	Automobile, station wagon	Animal - wild	Driving properly	
Comments	: animal							9			



From: January 1, 2017 To: August 22, 2022

Location Thirty Road btwn Young Street & Clayson Road Municipality....... WEST LINCOLN

Traffic Control.... Unknown Total Collisions.... 1

Collision ID	Date/Day/Time	Environment	Impact Type	Classification	Direction	Surface Cond'n	Vehicle Manoeuver	Vehicle type	First Event	Driver Action	No. Ped
1919721	2019-Mar-05, Tue,08:06	Snow	Approaching	Non-fatal injury	North	Ice	· ·	Automobile, station wagon	Other motor vehicle	Lost control	
Comments	s:				South		- 3	Automobile, station wagon	Other motor vehicle	Driving properly	

APPENDIX 3

All-Way Stop Control and Left-Turn Lane Warrant Analysis – Existing Conditions





OTM Arterial/Major Intersection All-Way Stop Warrant Analysis

Intersection: Thirty Road & Young Street

Major Roadway: Thirty Road

Minor Roadway: Young Street

Analysis Scenario: Existing (2022) Conditions

Analysis Period: 8 Hours

T-Intersection: No

				Traffic & Ped	destrian Volumes			
Start Time	Major Roadway	Peds (Minor)	Minor Roadway	Peds (Major)	Minor Volume	Total Vehicular Volume	Volume Split	Interval Satisfied
7:00	325	0	128	0	128	453	70 / 30	×
8:00	360	0	125	0	125	485	75 / 25	×
11:00	257	0	88	1	89	345	75 / 25	×
12:00	248	0	112	0	112	360	70 / 30	×
13:00	220	0	133	0	133	353	60 / 40	×
15:00	364	0	163	0	163	527	70 / 30	×
16:00	397	0	177	0	177	574	70 / 30	×
17:00	309	0	137	0	137	446	70 / 30	×

AWS Warrant 80% Satisfied: No AWS Warrant 100% Satisfied: No

Notes:

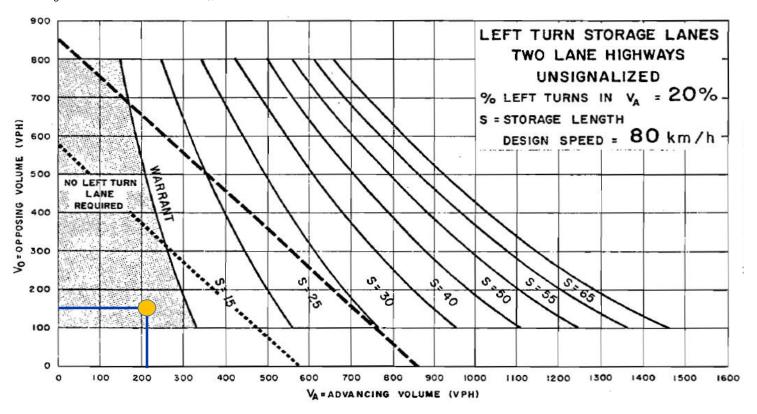
-Average delays to minor street traffic (vehicular and pedestrian) for the same eight hour period should exceed 30 seconds for AWS to be warranted.

-AWS control should also be considered where four or more right angle or turning collisions per year have occurred over a period of three years and other mitigation measures have been deemed inadequate.

LEFT-TURN WARRANT ANALYSIS

SCENARIO Existing Conditions PEAK HOUR PM Peak Hour MOVEMENT Northbound Left-turn

Number of Lanes 2-Lane Undivided **Design Speed Limit** 80 Advancing Traffic Volume 210 Opposing Traffic Volume 182 Left Turn Traffic Volume 39 Percentage of Left-Turn Traffic 20%



TRAFFIC SIGNALS MAY BE WARRANTED IN RURAL Opposing Volume AREAS OR URBAN AREAS WITH RESTRICTED FLOW TRAFFIC SIGNALS MAY BE WARRANTED IN

"FREE FLOW" URBAN AREAS

Advancing Volume

APPENDIX 4

Detailed Synchro Analysis Outputs – Existing Conditions



	٠	→	*	1	←	•	1	†	-	1	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	13	22	51	7	16	0	72	156	5	0	111	14
Future Volume (vph)	13	22	51	7	16	0	72	156	5	0	111	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	14	24	55	8	17	0	78	170	5	0	121	15
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	93	25	253	136								
Volume Left (vph)	14	8	78	0								
Volume Right (vph)	55	0	5	15								
Hadj (s)	-0.18	0.30	0.19	0.16								
Departure Headway (s)	4.7	5.2	4.6	4.6								
Degree Utilization, x	0.12	0.04	0.32	0.18								
Capacity (veh/h)	705	622	766	739								
Control Delay (s)	8.3	8.4	9.7	8.6								
Approach Delay (s)	8.3	8.4	9.7	8.6								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			9.1									
Level of Service			Α									
Intersection Capacity Utiliza	ition		36.9%	IC	U Level o	of Service			Α			
Analysis Period (min)			15									

Intersection			
Intersection Delay, s/veh	9		
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	13	22	51	7	16	0	72	156	5	0	111	14
Future Vol, veh/h	13	22	51	7	16	0	72	156	5	0	111	14
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	14	8	0	20	0	8	9	0	0	14	7
Mvmt Flow	14	24	55	8	17	0	78	170	5	0	121	15
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			1			1				1	
HCM Control Delay	8.1			8.2			9.6				8.6	
HCM LOS	Α			Α			Α				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	31%	15%	30%	0%	
Vol Thru, %	67%	26%	70%	89%	
Vol Right, %	2%	59%	0%	11%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	233	86	23	125	
LT Vol	72	13	7	0	
Through Vol	156	22	16	111	
RT Vol	5	51	0	14	
Lane Flow Rate	253	93	25	136	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.318	0.117	0.035	0.175	
Departure Headway (Hd)	4.515	4.508	4.982	4.627	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	798	795	718	776	
Service Time	2.539	2.538	3.018	2.654	
HCM Lane V/C Ratio	0.317	0.117	0.035	0.175	
HCM Control Delay	9.6	8.1	8.2	8.6	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	1.4	0.4	0.1	0.6	

	۶	→	←	4	1	1
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	13		W	
Traffic Volume (veh/h)	1	26	23	34	31	0
Future Volume (Veh/h)	1	26	23	34	31	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1	28	25	37	34	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		22	22			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	62				74	44
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	62				74	44
tC, single (s)	4.1				6.6	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.7	3.3
p0 queue free %	100				96	100
cM capacity (veh/h)	1554				889	1032
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total			34			
	29	62				
Volume Left	1	0	34			
Volume Right	0	37 4700	0			
cSH	1554	1700	889			
Volume to Capacity	0.00	0.04	0.04			
Queue Length 95th (m)	0.0	0.0	1.0			
Control Delay (s)	0.3	0.0	9.2			
Lane LOS	Α		Α			
Approach Delay (s)	0.3	0.0	9.2			
Approach LOS			Α			
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utilizat	tion		13.6%	IC	U Level o	of Service
Analysis Period (min)			15			

	1	•	†	~	/	ţ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT	I	
Lane Configurations		7	^			4		
Traffic Volume (veh/h)	0	35	169	0	31	125		
Future Volume (Veh/h)	0	35	169	0	31	125		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	0	38	184	0	34	136		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	388	184			184			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	388	184			184			
tC, single (s)	6.4	6.3			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.4			2.2			
p0 queue free %	100	95			98			
cM capacity (veh/h)	604	836			1385			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total		184	170					
	38							
Volume Left	0 38	0	34					
Volume Right cSH		1700	1205					
	836	1700	1385					
Volume to Capacity	0.05 1.1	0.11	0.02					
Queue Length 95th (m)		0.0	0.6					
Control Delay (s)	9.5	0.0	1.7					
Lane LOS	Α	0.0	A					
Approach Delay (s)	9.5	0.0	1.7					
Approach LOS	Α							
Intersection Summary								
Average Delay			1.7					
Intersection Capacity Utilization	n		25.3%	IC	U Level o	of Service		
Analysis Period (min)			15					

ntersection	
Intersection Delay, s/veh	9.2
Intersection LOS	Α

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	14	42	84	4	23	2	39	159	12	0	164	18
Future Vol, veh/h	14	42	84	4	23	2	39	159	12	0	164	18
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	0	1	0	4	0	5	3	0	0	4	6
Mvmt Flow	15	46	91	4	25	2	42	173	13	0	178	20
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			1			1				1	
HCM Control Delay	8.7			8.4			9.6				9.2	
HCM LOS	Α			Α			Α				Α	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	19%	10%	14%	0%	
Vol Thru, %	76%	30%	79%	90%	
Vol Right, %	6%	60%	7%	10%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	210	140	29	182	
LT Vol	39	14	4	0	
Through Vol	159	42	23	164	
RT Vol	12	84	2	18	
Lane Flow Rate	228	152	32	198	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.295	0.194	0.044	0.253	
Departure Headway (Hd)	4.649	4.586	5.073	4.608	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	772	779	702	777	
Service Time	2.688	2.63	3.13	2.648	
HCM Lane V/C Ratio	0.295	0.195	0.046	0.255	
HCM Control Delay	9.6	8.7	8.4	9.2	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	1.2	0.7	0.1	1	

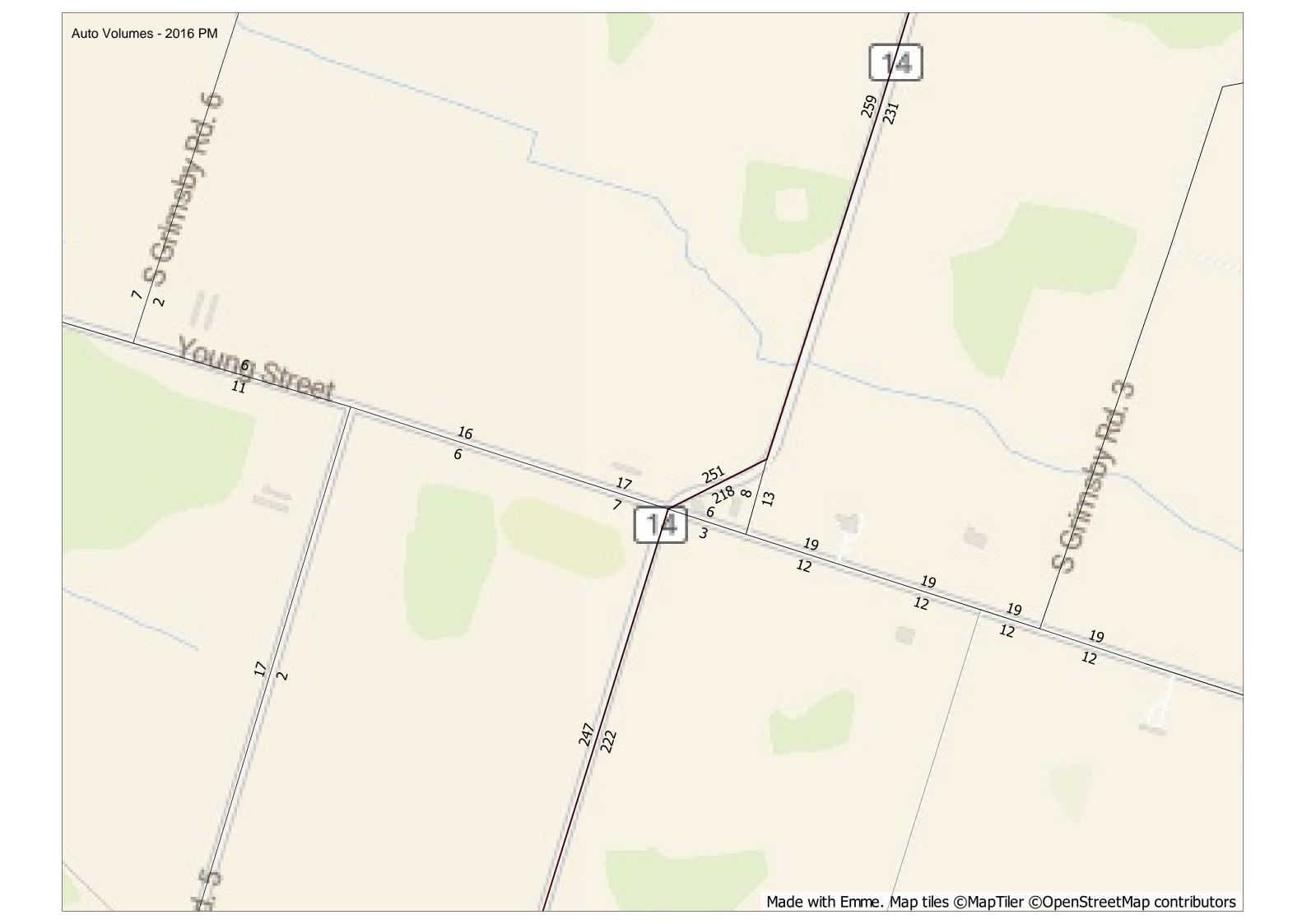
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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	1		**	
Traffic Volume (veh/h)	0	54	28	36	39	1
Future Volume (Veh/h)	0	54	28	36	39	1
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	59	30	39	42	1
Pedestrians						•
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		140110	140110			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	69				108	50
vC1, stage 1 conf vol	03				100	30
vC2, stage 2 conf vol						
vCu, unblocked vol	69				108	50
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	4.1				0.4	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	100				95	100
	1545				886	1025
cM capacity (veh/h)					000	1025
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	59	69	43			
Volume Left	0	0	42			
Volume Right	0	39	1			
cSH	1545	1700	889			
Volume to Capacity	0.00	0.04	0.05			
Queue Length 95th (m)	0.0	0.0	1.2			
Control Delay (s)	0.0	0.0	9.3			
Lane LOS			Α			
Approach Delay (s)	0.0	0.0	9.3			
Approach LOS			Α			
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization	n		14.0%	IC	ULevelo	of Service
Analysis Period (min)	·		15			22.1.00

	•	•	†	~	-	↓	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations		7	^			र्स	
Traffic Volume (veh/h)	0	36	175	0	40	182	
Future Volume (Veh/h)	0	36	175	0	40	182	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	39	190	0	43	198	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	474	190			190		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	474	190			190		
tC, single (s)	6.4	6.3			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.4			2.2		
p0 queue free %	100	95			97		
cM capacity (veh/h)	535	837			1378		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	39	190	241				
Volume Left	0	0	43				
Volume Right	39	0	0				
cSH	837	1700	1378				
Volume to Capacity	0.05	0.11	0.03				
Queue Length 95th (m)	1.2	0.0	8.0				
Control Delay (s)	9.5	0.0	1.6				
Lane LOS	Α		Α				
Approach Delay (s)	9.5	0.0	1.6				
Approach LOS	Α						
Intersection Summary							
Average Delay			1.6				
Intersection Capacity Utilization	on		29.5%	IC	U Level c	of Service	
Analysis Period (min)			15				

APPENDIX 5 EMME Model Outputs











APPENDIX 6

Alternative Solutions



Alternative 1: Do Nothing

Intersection remains as is, with no improvements (prior to all-way stop temporary condition). Required to be considered as part of the EA planning & design process.

- ➤ Does not address insufficient stopping sightline distance caused by reverse curve alignment
- Does not address right-of-way conflict at Thirty Road / Clayson Road
- Does not address westbound traffic utilizing Clayson Road as cut-through







Alternative 2: Intersection Improvements & Close Clayson Road at Northern Terminus

Close Clayson Road and direct traffic to improved Thirty Road at Young Street intersection i.e., all-way stop, roundabout or traffic signal (to be confirmed in Phase 3).

- ✓ Addresses insufficient stopping sightline distance caused by reverse curve alignment at Young Street / Thirty Road (dependent on intersection control selected in Phase 3)
- ✓ Eliminates right-of-way conflict at Thirty Road / Clayson Road
- ✓ Addresses east bound traffic utilizing Clayson Road as cut-through
- √ N/S traffic flow is maintained
- ✓ Minimal impact to adjacent lands





Alternative 3: Improvements at Clayson Rd & Young St Intersections & Close Thirty Road Segment

Close north segment of Thirty Road and direct traffic to Thirty Road at Clayson Road Street intersection.

- ✓ Eliminates stopping sightline distance issues at Young Street / Thirty Road
- ✓ Removes right-of-way conflict at Thirty Road and Clayson Road
- Minimal-moderate property requirements (to be determined based on type of intersection control)
- Results in minor delays to N/S traffic flow
- Requires upgrades to Clayson Road and Young Street

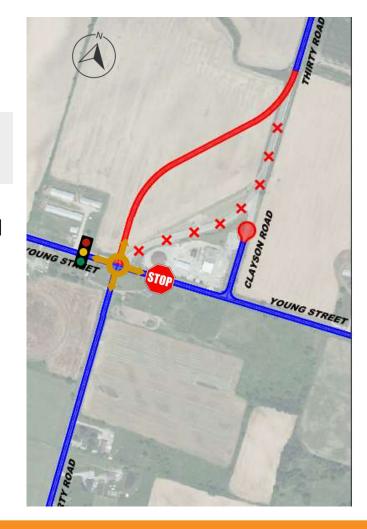




Alternative 4: Realign Thirty Road North of Young Street

Segment of Thirty Road north of Young Street is closed and realigned with Clayson Road closed at northern terminus.

- √ Improves stopping sightline distance at Young Street/Thirty Road
- ✓ Addresses right-of-way conflict at Thirty Road / Clayson Road
- ✓ Maintains N/S traffic flow
- ➤ Impacts watercourse and highly vulnerable aquifer
- Significant property impacts (bisects property north of Young)
- ➤ High cost to implement



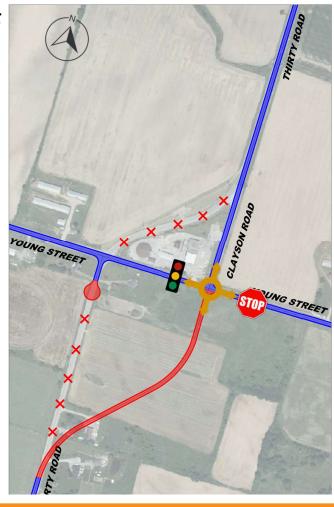


Alternative 5: Extend Clayson Road South of Young Street

Clayson Road is extended south of Young Street. Segment of Thirty Road north of Young Street is closed and Thirty Road is cul-de-sac'd at southern terminus.

- ✓ Addresses stopping sightline distance issue at Young Street / Thirty Road
- ✓ Removes right-of-way conflict at Thirty Road / Clayson Road
- ✓ Maintains N/S traffic flow
- Significant property impacts including potential cultural heritage property (farm south of Young and 3049 Thirty Road)
- ➤ High cost to implement





APPENDIX 7

All-Way Stop Control, Left-Turn Lane, and Traffic Signal Warrant Analysis – Future (2041) Traffic Conditions





OTM Arterial/Major Intersection All-Way Stop Warrant Analysis

Intersection: Thirty Road & Young Street

Major Roadway: Thirty Road

Minor Roadway: Young Street

Analysis Scenario: Future (2042) Conditions (Alternative 2)

Analysis Period: 8 Hours

T-Intersection: No

	Traffic & Pedestrian Volumes													
Start Time	Major Roadway	Peds (Minor)	Minor Roadway	Peds (Major)	Minor Volume	Total Vehicular Volume	Volume Split	Interval Satisfied						
7:00	520	0	205	0	205	725	70 / 30	✓						
8:00	576	0	200	0	200	776	75 / 25	×						
11:00	411	0	141	1	142	552	75 / 25	×						
12:00	396	0	179	0	179	575	70 / 30	×						
13:00	352	0	213	0	213	565	60 / 40	✓						
15:00	582	0	261	0	261	843	70 / 30	✓						
16:00	635	0	283	0	283	918	70 / 30	✓						
17:00	494	0	219	0	219	713	70 / 30	✓						

AWS Warrant 80% Satisfied: No AWS Warrant 100% Satisfied: No

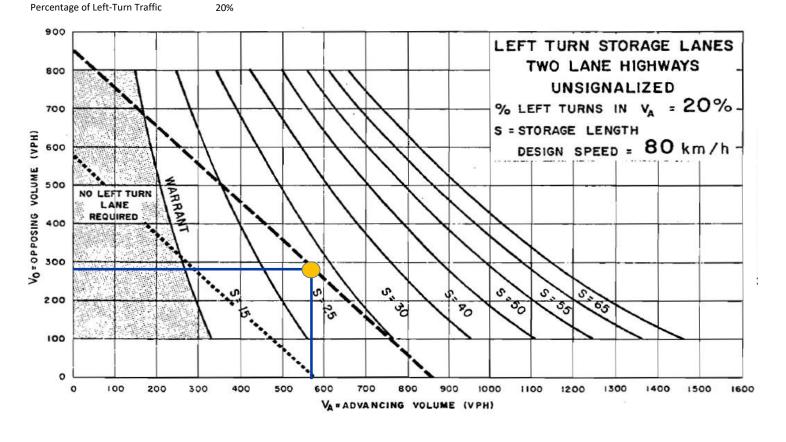
Notes:

-Average delays to minor street traffic (vehicular and pedestrian) for the same eight hour period should exceed 30 seconds for AWS to be warranted.

-AWS control should also be considered where four or more right angle or turning collisions per year have occurred over a period of three years and other mitigation measures have been deemed inadequate.

LEFT-TURN WARRANT ANALYSIS

SCENARIO Future (2041) Conditions **PEAK HOUR** AM Peak Hour MOVEMENT Northbound Left-turn Number of Lanes 2-Lane Undivided **Design Speed Limit** 80 Advancing Traffic Volume 575 Opposing Traffic Volume 249 Left Turn Traffic Volume 115



TRAFFIC SIGNALS MAY BE WARRANTED IN RURAL
AREAS OR URBAN AREAS WITH RESTRICTED FLOW

TRAFFIC SIGNALS MAY BE WARRANTED IN

"FREE FLOW" URBAN AREAS

TRAFFIC SIGNALS MAY BE WARRANTED IN

"FREE FLOW" URBAN AREAS

OTM Book 12 - Signal Warrant Analysis - Justification 7 - Projected Volumes

Intersection: Thirty Road & Young Street

Major Roadway:	Thirty Road	Orientation: East-West	No. of Lanes: 1
Minor Roadway	Young Street	Orientation: North-South	No. of Lanes: 1

Analysis Scenario: Future Total (2042) Traffic Conditions (Alternative 2)

Peak Hour¹: AM & PM Average Hour Volumes

Existing Intersection¹: Yes

T-Intersection²: No Signal Warrant 150% Satisfied: No Flow Conditions³: Free Flow Signal Warrant 120% Satisfied: No

Traffic & Pedestrian Volumes

					Major F	Roadway				Minor Roadway								
Peak Hour		North	bound			South	bound				Eastb	ound			West	bound		
	Left	Thru	Right	Total	Left	Thru	Right	Total	Peds	Left	Thru	Right	Total	Left	Thru	Right	Total	Peds
АМ	115	452	8	575	22	178	0	200	0	21	36	82	139	11	29	0	40	0
PM	62	349	19	430	29	351	0	380	0	22	68	134	224	6	38	3	47	0
Totals	177	801	27	1005	51	529	0	580	0	43	104	216	363	17	67	3	87	0

Table 21 - Justification 7 - Projected Volumes

		Minimum Requ	uirement 1 Lane	Minimum Requir	rement 2 or more		Compliance	
Justification	Description	High	iways	lar	nes	Sect	ional	
		Free Flow	Restr. Flow	Free Flow	Restr. Flow	Numerical	%	Entire %
Minimum Vehicular Volume	A. Vehicle volume, all approaches (average hour)	480	720	600	900	509	106%	94%
1. Willimum Venicular Volume	B. Vehicle volume, along minor streets * (average hour)	120	170	120	170	113	94%	94%
	A. Vehicle volume, major street (average hour)	480	720	600	900	396	83%	
2. Delay to Cross Traffic	B. Combined vehicle and pedestrian volume crossing artery from minor streets (average hour)	50	75	120	170	41	82%	82%

Notes:

- 1. If using Average Hour Volumes, Justifications 1 and 2 should be met to 120% for an existing intersection or 150% for a new intersection.
- 2. For "T" intersections, these values should be increased by 50%
- 3. Restricted flow: Operating or posted speed less than 70km/hr. Normally urban areas.

Free flow: Operating or posted speeds equal to or greater than 70km/hr. Normally rural areas.

- Application of Justification 2b:
 - The number of pedestrians crossing the main road.
 - Total left turns from both the side road approaches.
 - The highest through volumes from one of the side road approaches.
 - Fifty percent of the heavier left-turn traffic movement from the main road when both of the following criteria are met:
 - The left turn volume is greater than 120 vehicles per hour.
 - The total of the heavier left-turn volume plus its opposing volume is greater than 720 vph

APPENDIX 8

Detailed Synchro Analysis Outputs – Future (2041) Traffic Conditions – Alternative 1



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	21	35	82	11	29	0	115	452	8	0	178	22
Future Vol, veh/h	21	35	82	11	29	0	115	452	8	0	178	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	14	8	0	20	0	8	9	0	0	14	7
Mvmt Flow	23	38	89	12	32	0	125	491	9	0	193	24
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			1			1				1	
HCM Control Delay	10.7			10.1			31.4				11.3	
HCM LOS	В			В			D				В	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	20%	15%	28%	0%	
Vol Thru, %	79%	25%	72%	89%	
Vol Right, %	1%	59%	0%	11%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	575	138	40	200	
LT Vol	115	21	11	0	
Through Vol	452	35	29	178	
RT Vol	8	82	0	22	
Lane Flow Rate	625	150	43	217	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.866	0.242	0.078	0.333	
Departure Headway (Hd)	4.987	5.798	6.443	5.507	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	725	618	554	652	
Service Time	3.013	3.845	4.502	3.546	
HCM Lane V/C Ratio	0.862	0.243	0.078	0.333	
HCM Control Delay	31.4	10.7	10.1	11.3	
HCM Lane LOS	D	В	В	В	
HCM 95th-tile Q	10.4	0.9	0.3	1.5	

	۶	→	←	4	1	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1→		W	
Traffic Volume (veh/h)	2	42	40	56	50	0
Future Volume (Veh/h)	2	42	40	56	50	0
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	46	43	61	54	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		, <u>.</u>				
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	104				124	74
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	104				124	74
tC, single (s)	4.1				6.6	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.7	3.3
p0 queue free %	100				94	100
cM capacity (veh/h)	1500				831	994
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	48	104	54			
Volume Left	2	0	54			
Volume Right	0	61	0			
cSH	1500	1700	831			
Volume to Capacity	0.00	0.06	0.06			
Queue Length 95th (m)	0.00	0.00	1.7			
Control Delay (s)	0.0	0.0	9.6			
Lane LOS	0.3 A	0.0	9.0 A			
Approach Delay (s)	0.3	0.0	9.6			
Approach LOS	0.5	0.0	9.0 A			
			А			
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utiliza	ation		16.0%	IC	U Level o	of Service
Analysis Period (min)			15			

	1	*	†	~	-	ļ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	↑			र्स
Traffic Volume (veh/h)	0	56	473	0	50	200
Future Volume (Veh/h)	0	56	473	0	50	200
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	61	514	0	54	217
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	839	514			514	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	839	514			514	
tC, single (s)	6.4	6.3			4.3	
tC, 2 stage (s)						
tF (s)	3.5	3.4			2.4	
p0 queue free %	100	89			94	
cM capacity (veh/h)	320	543			970	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	61	514	271			
Volume Left	0	0	54			
Volume Right	61	0	0			
cSH	543	1700	970			
Volume to Capacity	0.11	0.30	0.06			
Queue Length 95th (m)	3.0	0.0	1.4			
Control Delay (s)	12.5	0.0	2.2			
Lane LOS	12.3 B	0.0	Z.Z A			
Approach Delay (s)	12.5	0.0	2.2			
Approach LOS	12.3 B	0.0	۷.۷			
	D					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utiliz	zation		48.1%	IC	U Level c	of Service
Analysis Period (min)			15			

Intersection												
Intersection Delay, s/veh	18.4											
Intersection LOS	С											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			1			412			4	

Movement	EBL	EBI	EBK	WBL	WBI	WBK	INBL	INRT	INBK	SBL	SBI	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	22	68	134	6	38	3	62	349	19	0	351	29
Future Vol, veh/h	22	68	134	6	38	3	62	349	19	0	351	29
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	0	1	0	4	0	5	3	0	0	4	6
Mvmt Flow	24	74	146	7	41	3	67	379	21	0	382	32
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB				SB	
Opposing Approach	WB			EB			SB				NB	
Opposing Lanes	1			1			1				1	
Conflicting Approach Left	SB			NB			EB				WB	
Conflicting Lanes Left	1			1			1				1	
Conflicting Approach Right	NB			SB			WB				EB	
Conflicting Lanes Right	1			1			1				1	
HCM Control Delay	13.2			10.8			22				18.3	
HCM LOS	В			В			С				С	

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	14%	10%	13%	0%	
Vol Thru, %	81%	30%	81%	92%	
Vol Right, %	4%	60%	6%	8%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	430	224	47	380	
LT Vol	62	22	6	0	
Through Vol	349	68	38	351	
RT Vol	19	134	3	29	
Lane Flow Rate	467	243	51	413	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.724	0.406	0.099	0.642	
Departure Headway (Hd)	5.576	6.001	7.004	5.596	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	645	594	515	640	
Service Time	3.652	4.095	5.004	3.675	
HCM Lane V/C Ratio	0.724	0.409	0.099	0.645	
HCM Control Delay	22	13.2	10.8	18.3	
HCM Lane LOS	С	В	В	С	
HCM 95th-tile Q	6.1	2	0.3	4.6	

	۶	→	←	1	1	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1→		**	
Traffic Volume (veh/h)	0	87	45	58	64	2
Future Volume (Veh/h)	0	87	45	58	64	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	95	49	63	70	2
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110	110110			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	112				176	80
vC1, stage 1 conf vol	112				.,,	30
vC2, stage 2 conf vol						
vCu, unblocked vol	112				176	80
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	7.1				J. T	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	100				91	100
cM capacity (veh/h)	1490				812	985
		14/E (05 1		012	300
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	95	112	72			
Volume Left	0	0	70			
Volume Right	0	63	2			
cSH	1490	1700	816			
Volume to Capacity	0.00	0.07	0.09			
Queue Length 95th (m)	0.0	0.0	2.3			
Control Delay (s)	0.0	0.0	9.8			
Lane LOS			Α			
Approach Delay (s)	0.0	0.0	9.8			
Approach LOS			Α			
Intersection Summary						
Average Delay			2.5			
Intersection Capacity Utilization	on		17.1%	IC	U Level o	f Service
Analysis Period (min)			15			

	1	•	†	~	1	ļ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	^			4
Traffic Volume (veh/h)	0	58	374	0	64	380
Future Volume (Veh/h)	0	58	374	0	64	380
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	63	407	0	70	413
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	960	407			407	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	960	407			407	
tC, single (s)	6.4	6.3			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.4			2.2	
p0 queue free %	100	90			94	
cM capacity (veh/h)	270	631			1146	
		NB 1	SB 1			
Direction, Lane #	WB 1					
Volume Total	63	407	483			
Volume Left	0	0	70			
Volume Right	63	0	0			
cSH	631	1700	1146			
Volume to Capacity	0.10	0.24	0.06			
Queue Length 95th (m)	2.6	0.0	1.6			
Control Delay (s)	11.3	0.0	1.8			
Lane LOS	В		A			
Approach Delay (s)	11.3	0.0	1.8			
Approach LOS	В					
Intersection Summary						
Average Delay			1.7			
Intersection Capacity Utilization	on		53.6%	IC	U Level o	of Service
Analysis Period (min)			15			

APPENDIX 9

Detailed Synchro and Arcady Analysis Outputs – Future (2041) Traffic Conditions – Alternatives 2, 4 and 5



Intersection												
Intersection Delay, s/veh	31.4											
Intersection LOS	D											
Movement	FBI	FBT	FBR	WBI	WRT	WBR	NBI	NBT	NBR	SBI	SBT	SBF

Movement	EBL	FBT	EBK	WBL	WBI	WBR	NBL	NRI	NBK	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	51	35	82	11	26	59	115	452	8	50	177	22
Future Vol, veh/h	51	35	82	11	26	59	115	452	8	50	177	22
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	0	14	8	0	19	9	8	9	0	19	14	7
Mvmt Flow	55	38	89	12	28	64	125	491	9	54	192	24
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	12.7			11.2			47.4			14.7		
HCM LOS	В			В			Е			В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	20%	30%	11%	20%	
Vol Thru, %	79%	21%	27%	71%	
Vol Right, %	1%	49%	61%	9%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	575	168	96	249	
LT Vol	115	51	11	50	
Through Vol	452	35	26	177	
RT Vol	8	82	59	22	
Lane Flow Rate	625	183	104	271	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.95	0.329	0.192	0.471	
Departure Headway (Hd)	5.575	6.478	6.613	6.259	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	656	557	544	579	
Service Time	3.575	4.499	4.64	4.259	
HCM Lane V/C Ratio	0.953	0.329	0.191	0.468	
HCM Control Delay	47.4	12.7	11.2	14.7	
HCM Lane LOS	Е	В	В	В	
HCM 95th-tile Q	13.3	1.4	0.7	2.5	

Intersection												
Intersection Delay, s/veh	27											
Intersection LOS	D											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	22	67	134	6	38	61	62	348	19	67	351	35
Future Vol, veh/h	22	67	134	6	38	61	62	348	19	67	351	35
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	٥	Λ	1	٥	4	8	5	3	17	3	1	5

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Mvmt Flow	24	73	146	7	41	66	67	378	21	73	382	38
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	15.3			12.5			30.2			33.2		
HCM LOS	С			В			D			D		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	14%	10%	6%	15%	
Vol Thru, %	81%	30%	36%	77%	
Vol Right, %	4%	60%	58%	8%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	429	223	105	453	
LT Vol	62	22	6	67	
Through Vol	348	67	38	351	
RT Vol	19	134	61	35	
Lane Flow Rate	466	242	114	492	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.805	0.453	0.231	0.837	
Departure Headway (Hd)	6.212	6.721	7.299	6.119	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	578	533	495	591	
Service Time	4.288	4.809	5.299	4.195	
HCM Lane V/C Ratio	0.806	0.454	0.23	0.832	
HCM Control Delay	30.2	15.3	12.5	33.2	
HCM Lane LOS	D	С	В	D	
HCM 95th-tile Q	7.9	2.3	0.9	8.8	

Junctions 10

ARCADY 10 - Roundabout Module

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Filename: 226468-Arcady Analysis.j10 **Path:** C:\Users\arcady\Desktop

Report generation date: 2023-05-23 10:12:44 AM

«Roundabout Analysis - 2042, AM

- **»Junction Network**
- »Arms
- »Traffic Demand
- »Origin-Destination Data
- »Vehicle Mix
- »Detailed Demand Data
- »Results
- »Graphs

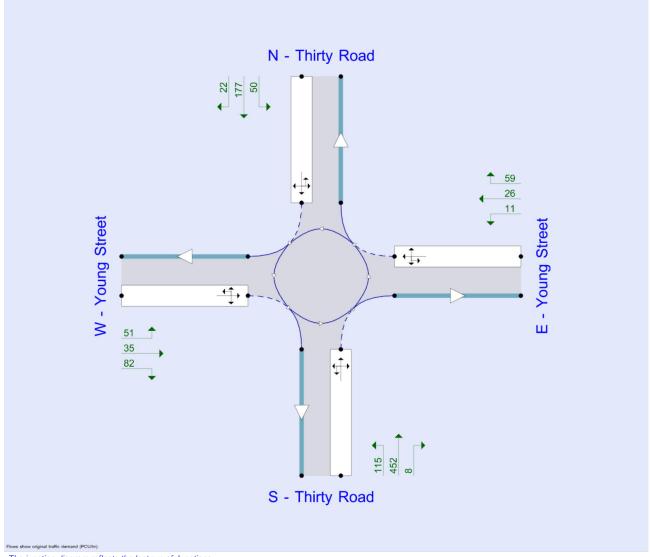
File summary

File Description

Title	
Location	
Site number	
Date	2023-03-03
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	RVAINT\arcady
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	s	-Min	perMin



The junction diagram reflects the last run of Junctions.

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
✓		0.85	36.00	20.00

HCM Calibration

HCM Calibration	Lane type	Num circulating lanes	Num exit lanes	Α	В
1	Single lane	1		1380.00	-0.00102
2	Single lane	2		1420.00	-0.00085
3	Nearside	1		1420.00	-0.00091
4	Nearside	2		1420.00	-0.00085
5	Offside	1		1420.00	-0.00091
6	Offside	2		1350.00	-0.00092
7	Yielding bypass		1	1380.00	-0.00102
8	Yielding bypass		2	1420.00	-0.00085
9	Non-yielding bypass		1	99999.00	0.00000

Analysis Set Details

ID	Name	Network flow scaling factor (%)
A1	Roundabout Analysis	100.000

Demand Set Details

Roundabout Analysis - 2042, AM

Data Errors and Warnings

Severity Area Item Description		Description			
Warning HCM Model D1 - 2042, AM Demand Set 1: HCM models are most typically used with PHF traffic flow profiles a segments. Use of HCM models with other flow profiles is at the user's own risk		Demand Set 1: HCM models are most typically used with PHF traffic flow profiles and single time segments. Use of HCM models with other flow profiles is at the user's own risk			
Warning			One or more junctions use HCM methodologies. These methods are not associated with TRL. The user should apply judgement when interpreting the results.		
Warning Queue variations Analysis Options		Analysis Options	Queue percentiles may be unreliable if the mean queue in any time segment is very low or very high.		

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS	
1	Thirty Road & Young Street	HCM Roundabout		E, N, W, S	7.82	А	

Junction Network

Driving side	Lighting	Network delay (s)	Network LOS
Right	Normal/unknown	7.82	Α

Arms

Arms

Arm	Name	Description
E	Young Street	
N	Thirty Road	
w	Young Street	
S	Thirty Road	

HCM Lanes

Arm	HCM Lane	Lane type	Number of conflicting lanes	Destination arms
E - Young Street	1	Single lane	1	E, N, W, S
N - Thirty Road	1	Single lane	1	E, N, W, S
W - Young Street	1	Single lane	1	E, N, W, S
S - Thirty Road	1	Single lane	1	E, N, W, S

Traffic Demand

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

	•	•		
Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
E - Young Street		✓	96	100.000
N - Thirty Road		✓	249	100.000
W - Young Street		✓	168	100.000
S - Thirty Road		✓	575	100.000

Peak Hour Factor Data (Traffic)

Arm	Hourly volume (PCU/hr)	Peak hour factor	Peak time segment
E - Young Street	96	0.92	SecondQuarter
N - Thirty Road	249	0.92	SecondQuarter

W - Young Street	168	0.92	SecondQuarter
S - Thirty Road	575	0.92	SecondQuarter

Origin-Destination Data

Demand (PCU/hr)

	То						
		E - Young Street	N - Thirty Road	W - Young Street	S - Thirty Road		
	E - Young Street	0	59	26	11		
From	N - Thirty Road	50	0	22	177		
	W - Young Street	35	51	0	82		
	S - Thirty Road	8	452	115	0		

Vehicle Mix

Heavy Vehicle Percentages

	То						
		E - Young Street	N - Thirty Road	W - Young Street	S - Thirty Road		
	E - Young Street	0	9	19	0		
From	N - Thirty Road	19	0	7	14		
	W - Young Street	14	0	0	8		
	S - Thirty Road	0	9	8	0		

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/hr)	Demand in PCU (PCU/hr)	Pedestrian Demand (Ped/hr)
	E - Young Street	90	90	0.00
08:00-08:15	N - Thirty Road	235	235	0.00
08.00-08.13	W - Young Street	158	158	0.00
	S - Thirty Road	542	542	0.00
	E - Young Street	104	104	0.00
08:15-08:30	N - Thirty Road	271	271	0.00
	W - Young Street	183	183	0.00
	S - Thirty Road	625	625	0.00
	E - Young Street	99	99	0.00
08:30-08:45	N - Thirty Road	256	256	0.00
06:30-06:45	W - Young Street	173	173	0.00
	S - Thirty Road	592	592	0.00
	E - Young Street	90	90	0.00
08:45-09:00	N - Thirty Road	235	235	0.00
00.45-09:00	W - Young Street	158	158	0.00
	S - Thirty Road	542	542	0.00

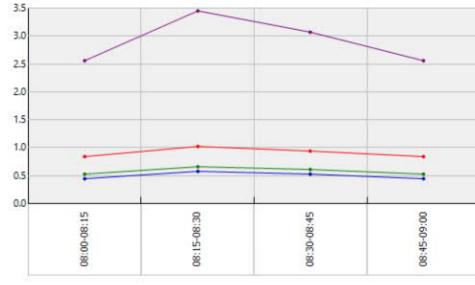
Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max 95th percentile Queue (PCU)	Max LOS			
E - Young Street	0.15	7.47	0.6	А			
N - Thirty Road	0.23	5.75	1.0	A			
W - Young Street	0.17	5.23	0.7	А			
S - Thirty Road	0.53	9.53	3.5	А			

Graphs





Junctions 10

ARCADY 10 - Roundabout Module

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Path: R:\2022\226468 - Niagara-Thirty(Rd) at Young St Class EA\08 Design\11 Transportation

Planning\Analysis\Alternatives\4 - Thirty Closure\Arcady Report generation date: 2023-05-23 10:22:58 AM

«Roundabout Analysis - 2042, PM

- »Junction Network
- »Arms
- »Traffic Demand
- »Origin-Destination Data
- »Vehicle Mix
- »Detailed Demand Data
- »Results
- » Graphs

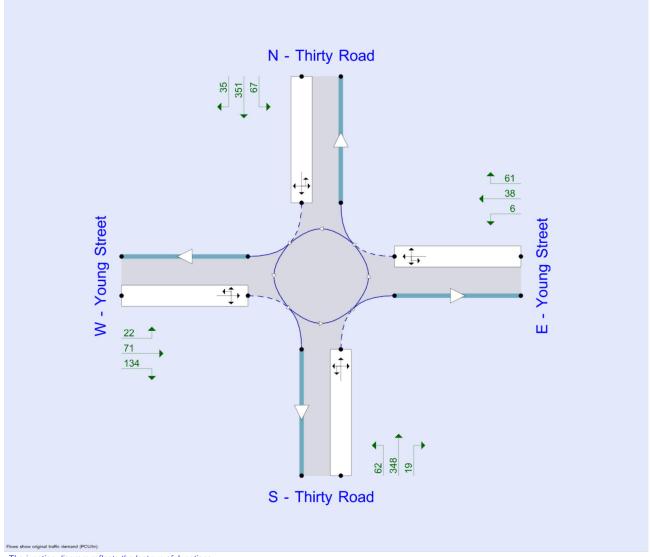
File summary

File Description

Title			
Location			
Site number			
Date	2023-03-03		
Version			
Status	(new file)		
Identifier			
Client			
Jobnumber			
Enumerator	RVAINT\arcady		
Description			

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	s	-Min	perMin



The junction diagram reflects the last run of Junctions.

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
✓		0.85	36.00	20.00

HCM Calibration

HCM Calibration	Lane type	Num circulating lanes	Num exit lanes	Α	В
1	Single lane	1		1380.00	-0.00102
2	Single lane	2		1420.00	-0.00085
3	Nearside	1		1420.00	-0.00091
4	Nearside	2		1420.00	-0.00085
5	Offside	1		1420.00	-0.00091
6	Offside	2		1350.00	-0.00092
7	Yielding bypass		1	1380.00	-0.00102
8	Yielding bypass		2	1420.00	-0.00085
9	Non-yielding bypass		1	99999.00	0.00000

Analysis Set Details

ID	Name	Network flow scaling factor (%)
A1	Roundabout Analysis	100.000

Demand Set Details

Roundabout Analysis - 2042, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	HCM Model	D2 - 2042, PM	Demand Set 2: HCM models are most typically used with PHF traffic flow profiles and single time segments. Use of HCM models with other flow profiles is at the user's own risk
Warning	HCM Model		One or more junctions use HCM methodologies. These methods are not associated with TRL. The user should apply judgement when interpreting the results.
Warning	Queue variations	Analysis Options	Queue percentiles may be unreliable if the mean queue in any time segment is very low or very high.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS
1	Thirty Road & Young Street	HCM Roundabout		E, N, W, S	7.14	Α

Junction Network

Driving side	Lighting	Network delay (s)	Network LOS
Right	Normal/unknown	7.14	Α

Arms

Arms

Arm	Name	Description
E	Young Street	
N	Thirty Road	
W	Young Street	
S	Thirty Road	

HCM Lanes

Arm	HCM Lane	Lane type	Number of conflicting lanes	Destination arms
E - Young Street	1	Single lane	1	E, N, W, S
N - Thirty Road	1	Single lane	1	E, N, W, S
W - Young Street	1	Single lane	1	E, N, W, S
S - Thirty Road	1	Single lane	1	E, N, W, S

Traffic Demand

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
E - Young Street		✓	105	100.000
N - Thirty Road		✓	453	100.000
W - Young Street		✓	227	100.000
S - Thirty Road		✓	429	100.000

Peak Hour Factor Data (Traffic)

Arm	Hourly volume (PCU/hr)	Peak hour factor	Peak time segment
E - Young Street	105	0.92	SecondQuarter
N - Thirty Road	453	0.92	SecondQuarter

W - Young Street	227	0.92	SecondQuarter
S - Thirty Road	429	0.92	SecondQuarter

Origin-Destination Data

Demand (PCU/hr)

			То		
		E - Young Street	N - Thirty Road	W - Young Street	S - Thirty Road
	E - Young Street	0	61	38	6
From	N - Thirty Road	67	0	35	351
	W - Young Street	71	22	0	134
	S - Thirty Road	19	348	62	0

Vehicle Mix

Heavy Vehicle Percentages

			То		
		E - Young Street	N - Thirty Road	W - Young Street	S - Thirty Road
	E - Young Street	0	8	4	0
From	N - Thirty Road	3	0	5	4
	W - Young Street	0	0	0	1
	S - Thirty Road	17	3	5	0

Detailed Demand Data

Demand for each time segment

Time Segment	Arm	Demand (PCU/hr)	Demand in PCU (PCU/hr)	Pedestrian Demand (Ped/hr)
	E - Young Street	99	99	0.00
00:00-00:15	N - Thirty Road	427	427	0.00
00.00-00.15	W - Young Street	214	214	0.00
	S - Thirty Road	404	404	0.00
	E - Young Street	114	114	0.00
00:15-00:30	N - Thirty Road	492	492	0.00
00:15-00:30	W - Young Street	247	247	0.00
	S - Thirty Road	466	466	0.00
	E - Young Street	108	108	0.00
00:30-00:45	N - Thirty Road	466	466	0.00
00:30-00:45	W - Young Street	234	234	0.00
	S - Thirty Road	441	441	0.00
	E - Young Street	99	99	0.00
00:45-01:00	N - Thirty Road	427	427	0.00
00.45-01:00	W - Young Street	214	214	0.00
	S - Thirty Road	404	404	0.00

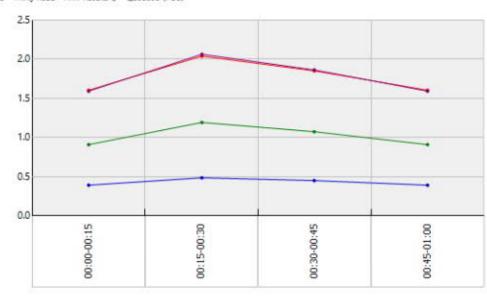
Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max 95th percentile Queue (PCU)	Max LOS
E - Young Street	0.13	5.82	0.5	А
N - Thirty Road	0.40	7.08	2.0	A
W - Young Street	0.29	7.30	1.2	A
S - Thirty Road	0.40	7.42	2.1	А

Graphs





APPENDIX 10

Detailed Synchro Analysis Outputs – Future (2041) Traffic Conditions – Alternative 3



	→	•	1	←	4	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f _è			र्स	W	
Traffic Volume (veh/h)	86	82	188	48	115	460
Future Volume (Veh/h)	86	82	188	48	115	460
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	93	89	204	52	125	500
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			182		598	138
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			182		598	138
tC, single (s)			4.2		6.5	6.3
tC, 2 stage (s)					U.U	
tF (s)			2.3		3.6	3.4
p0 queue free %			85		68	44
cM capacity (veh/h)			1330		386	893
	ED 4	WD 4				
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	182	256	625			
Volume Left	0	204	125			
Volume Right	89	0	500			
cSH	1700	1330	707			
Volume to Capacity	0.11	0.15	0.88			
Queue Length 95th (m)	0.0	4.3	88.2			
Control Delay (s)	0.0	6.8	36.0			
Lane LOS		Α	Е			
Approach Delay (s)	0.0	6.8	36.0			
Approach LOS			Е			
Intersection Summary						
Average Delay			22.8			
Intersection Capacity Utiliza	ation		67.2%	IC	U Level c	f Service
Analysis Period (min)	A.((-))		15	10	2 201010	501 1100
Analysis i ellou (IIIIII)			10			

	٠	→	•	*	-	4
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	1		W	
Traffic Volume (veh/h)	505	40	34	65	50	199
Future Volume (Veh/h)	505	40	34	65	50	199
Sign Control (Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	549	43	37	71	54	216
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	108				1214	72
vC1, stage 1 conf vol						. =
vC2, stage 2 conf vol						
vCu, unblocked vol	108				1214	72
tC, single (s)	4.2				6.6	6.3
tC, 2 stage (s)					3.0	J.0
tF (s)	2.3				3.7	3.4
p0 queue free %	62				53	77
cM capacity (veh/h)	1446				115	960
		MD 4	0D 4			
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	592	108	270			
Volume Left	549	0	54			
Volume Right	0	71	216			
cSH	1446	1700	389			
Volume to Capacity	0.38	0.06	0.69			
Queue Length 95th (m)	14.5	0.0	40.7			
Control Delay (s)	8.6	0.0	32.8			
Lane LOS	Α		D			
Approach Delay (s)	8.6	0.0	32.8			
Approach LOS			D			
Intersection Summary						
Average Delay			14.4			
Intersection Capacity Utiliza	tion		58.5%	IC	U Level o	of Service
Analysis Period (min)			15			

	-	*	1	←	1	-	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	7+			4	N/		
Traffic Volume (veh/h)	89	134	357	73	62	367	
Future Volume (Veh/h)	89	134	357	73	62	367	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	97	146	388	79	67	399	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume			243		1025	170	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			243		1025	170	
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			70		63	54	
cM capacity (veh/h)			1312		181	869	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	243	467	466				
Volume Left	0	388	67				
Volume Right	146	0	399				
cSH	1700	1312	562				
Volume to Capacity	0.14	0.30	0.83				
Queue Length 95th (m)	0.0	10.0	68.2				
Control Delay (s)	0.0	7.9	35.2				
Lane LOS		Α	Е				
Approach Delay (s)	0.0	7.9	35.2				
Approach LOS			Е				
Intersection Summary							
Average Delay			17.1				
Intersection Capacity Utiliza	tion		72.6%	IC	U Level o	f Service	
Analysis Period (min)			15				

	۶	→	+	•	\	4	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્ન	ĵ.		14		
Traffic Volume (veh/h)	372	88	41	69	67	388	
Future Volume (Veh/h)	372	88	41	69	67	388	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	404	96	45	75	73	422	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	120				986	82	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	120				986	82	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF(s)	2.2				3.5	3.3	
p0 queue free %	72				63	57	
cM capacity (veh/h)	1462				198	972	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	500	120	495				
Volume Left	404	0	73				
Volume Right	0	75	422				
cSH	1462	1700	616				
Volume to Capacity	0.28	0.07	0.80				
Queue Length 95th (m)	9.1	0.07	64.1				
Control Delay (s)	7.3	0.0	30.4				
Lane LOS	A 7.2	0.0	D 20.4				
Approach LOS	7.3	0.0	30.4				
Approach LOS			D				
Intersection Summary							
Average Delay			16.8				
Intersection Capacity Utilizat	tion		66.2%	IC	U Level c	of Service	
Analysis Period (min)			15				

Junctions 10

ARCADY 10 - Roundabout Module

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The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the

Filename: 226468-Arcady Analysis-Scenario 4-Both Junctions.j10

Path: R:\2022\226468 - Niagara-Thirty(Rd) at Young St Class EA\08 Design\11 Transportation

Planning\Analysis\Alternatives\4 - Thirty Closure\Arcady Report generation date: 2023-05-23 3:46:26 PM

»2041, AM »2041, PM

Summary of junction performance

		АМ			PM					
	Set ID	95% Queue (PCU)	Delay (s)	RFC	Los	Set ID	95% Queue (PCU)	Delay (s)	RFC	Los
					20	41				
Thirty Road & Young Street (East Junction) - Young Street (W)		2.4	7.31	0.45	Α		1.9	6.69	0.39	Α
Thirty Road & Young Street (East Junction) - Young Street (E)		0.5	6.02	0.14	Α		0.5	5.49	0.13	Α
Thirty Road & Young Street (East Junction) - Thirty Road (N)	D1	0.8	4.44	0.20	Α	D2	1.8	6.41	0.38	Α
Thirty Road & Young Street (West Junction) - Young Street (E)	וט	0.9	5.30	0.21	Α	D2	1.7	6.37	0.36	Α
Thirty Road & Young Street (West Junction) - Young Street (W)		0.6	4.98	0.16	Α		1.1	6.63	0.27	Α
Thirty Road & Young Street (West Junction) - Thirty Road (S)		3.1	8.67	0.50	Α		1.8	6.68	0.37	Α

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

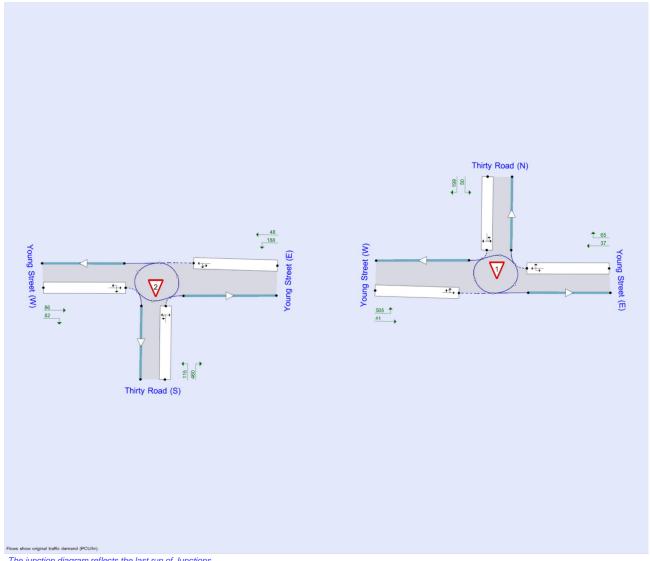
File summary

File Description

Title	
Location	
Site number	
Date	2023-03-31
Version	
Status	(new file)
Identifier	
Client	
Jobnumber	
Enumerator	RVAINT\arcady
Description	

Units

Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
m	kph	PCU	PCU	perHour	S	-Min	perMin



The junction diagram reflects the last run of Junctions.

Analysis Options

Calculate Queue Percentiles	Calculate residual capacity	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)
✓		0.85	36.00	20.00

HCM Calibration

HCM Calibration	Lane type	Num circulating lanes	Num exit lanes	Α	В
1	Single lane	1		1380.00	-0.00102
2	Single lane	2		1420.00	-0.00085
3	Nearside	1		1420.00	-0.00091
4	Nearside	2		1420.00	-0.00085
5	Offside	1		1420.00	-0.00091
6	Offside	2		1350.00	-0.00092
7	Yielding bypass		1	1380.00	-0.00102
8	Yielding bypass		2	1420.00	-0.00085
9	Non-yielding bypass		1	99999.00	0.00000

Demand Set Summary

ID	D Scenario name Time Period name		Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2041	AM	PHF	00:00	01:00	15
D2	2041	PM	PHF	00:00	01:00	15

2041, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	ing HCM Model D1 - 2041, AM		Demand Set 1: HCM models are most typically used with PHF traffic flow profiles and single time segments. Use of HCM models with other flow profiles is at the user's own risk
Warning	HCM Model	Thirty Road & Young Street (East Junction)	One or more junctions use HCM methodologies. These methods are not associated with TRL. The user should apply judgement when interpreting the results.
Warning	Vehicle Mix	Thirty Road & Young Street (East Junction)	HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs. If HV% at the junction is genuinely zero, please ignore this warning.
Warning	Queue variations	Analysis Options	Queue percentiles may be unreliable if the mean queue in any time segment is very low or very high.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS
1	Thirty Road & Young Street (East Junction)	HCM Roundabout		1, 2, 3	6.36	Α
2	Thirty Road & Young Street (West Junction)	HCM Roundabout		1, 2, 3	7.22	Α

Junction Network

Driving side Lighting		Network delay (s)	Network LOS	
Right	Normal/unknown	6.81	Α	

Arms

Arms

Junction	Arm	Name	Description
		Young Street (W)	
Thirty Road & Young Street (East Junction)	2	Young Street (E)	
	3	Thirty Road (N)	
	1	Young Street (E)	
Thirty Road & Young Street (West Junction)	2	Young Street (W)	
		Thirty Road (S)	

HCM Lanes

Junction	Arm	HCM Lane	Lane type	Number of conflicting lanes	Destination arms
	Young Street (W)	1	Single lane	1	1, 2, 3
Thirty Road & Young Street (East Junction)	Young Street (E)	1	Single lane	1	1, 2, 3
	Thirty Road (N)	1	Single lane	1	1, 2, 3
	Young Street (E)	1	Single lane	1	1, 2, 3
Thirty Road & Young Street (West Junction)	Young Street (W)	1	Single lane	1	1, 2, 3
	Thirty Road (S)	1	Single lane	1	1, 2, 3

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2041	AM	PHF	00:00	01:00	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Junction	Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
Thirty Road & Young Street (East Junction)	Young Street (W)		✓	546	100.000
	Young Street (E)		✓	102	100.000
	Thirty Road (N)		✓	249	100.000
Thirty Road & Young Street (West Junction)	Young Street (E)		✓	236	100.000
	Young Street (W)		✓	168	100.000
	Thirty Road (S)		✓	575	100.000

Peak Hour Factor Data (Traffic)

Junction	Arm	Hourly volume (PCU/hr)	Peak hour factor	Peak time segment	
Thirty Road & Young Street (East Junction)	Young Street (W)	546	0.92	SecondQuarter	
	Young Street (E)	102	0.92	SecondQuarter	
	Thirty Road (N)	249	0.92	SecondQuarter	
Thirty Road & Young Street (West Junction)	Young Street (E)	236	0.92	SecondQuarter	
	Young Street (W)	168	0.92	SecondQuarter	
	Thirty Road (S)	575	0.92	SecondQuarter	

Origin-Destination Data

Thirty Road & Young Street (East Junction)

Demand (PCU/hr)

	То						
		Young Street (W)	Young Street (E)	Thirty Road (N)			
F	Young Street (W)	0	41	505			
From	Young Street (E)	37	0	65			
	Thirty Road (N)	199	50	0			

Demand (PCU/hr)

Thirty Road & Young Street (West Junction)

	То						
		Young Street (E)		Thirty Road (S)			
From	Young Street (E)	0	48	188			
	Young Street (W)	86	0	82			
	Thirty Road (S)	460	115	0			

Vehicle Mix

Thirty Road & Young Street (East Junction)

Heavy Vehicle Percentages

	То						
		Young Street (W)	Young Street (E)	Thirty Road (N)			
From	Young Street (W)	0	0	0			
FIOIII	Young Street (E)	0	0	0			
	Thirty Road (N)	0	0	0			

Heavy Vehicle Percentages

Thirty Road & Young Street (West Junction)

	То						
		Young Street Y (E)		Thirty Road (S)			
From	Young Street (E)	0	13	13			
	Young Street (W)	9	0	8			
	Thirty Road (S)	9	8	0			

Detailed Demand Data

Demand for each time segment

Junction Arm Time Demand (PCU/hr) Demand in PCU Pedestrian Demand (PCU/hr) (PCU/hr) (Ped/hr)	Junction
--	----------

		00:00-00:15	514	514	0.00
	Young Street (W)	00:15-00:30	593	593	0.00
	roung Street (W)	00:30-00:45	562	562	0.00
		00:45-01:00	514	514	0.00
		00:00-00:15	96	96	0.00
Thirty Bood & Voung Street (East lungtion)	Vouna Street (E)	00:15-00:30	111	111	0.00
Thirty Road & Young Street (East Junction)	Young Street (E)	00:30-00:45	105	105	0.00
		00:45-01:00	96	96	0.00
		00:00-00:15	235	235	0.00
	Thirty Bood (N)	00:15-00:30	271	271	0.00
	Thirty Road (N)	00:30-00:45	256	256	0.00
		00:45-01:00	235	235	0.00
	Young Street (E)	00:00-00:15	222	222	0.00
		00:15-00:30	257	257	0.00
		00:30-00:45	243	243	0.00
		00:45-01:00	222	222	0.00
		00:00-00:15	158	158	0.00
Thirty Dood & Voung Street (Most Junetion)	Young Street (W)	00:15-00:30	183	183	0.00
Thirty Road & Young Street (West Junction)	roung Street (W)	00:30-00:45	173	173	0.00
		00:45-01:00	158	158	0.00
		00:00-00:15	542	542	0.00
	Thirty Road (S)	00:15-00:30	625	625	0.00
	Tillity Road (3)	00:30-00:45	592	592	0.00
		00:45-01:00	542	542	0.00

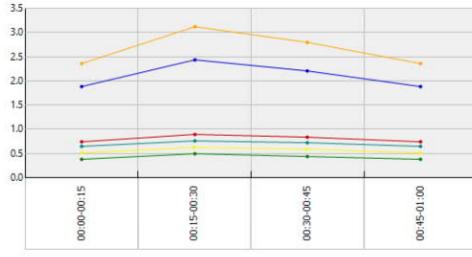
Results

Results Summary for whole modelled period

Junction	Arm	Max RFC	Max Delay (s)	Max 95th percentile Queue (PCU)	Max LOS
	Young Street (W)	0.45	7.31	2.4	Α
Thirty Road & Young Street (East Junction)	Young Street (E)	0.14	6.02	0.5	А
	Thirty Road (N)	0.20	4.44	0.8	А
	Young Street (E)	0.21	5.30	0.9	А
Thirty Road & Young Street (West Junction)	Young Street (W)	0.16	4.98	0.6	А
	Thirty Road (S)	0.50	8.67	3.1	А

Graphs





2041, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	HCM Model	D2 - 2041, PM	Demand Set 2: HCM models are most typically used with PHF traffic flow profiles and single time segments. Use of HCM models with other flow profiles is at the user's own risk
Warning	HCM Model	Thirty Road & Young Street (East Junction)	One or more junctions use HCM methodologies. These methods are not associated with TRL. The user should apply judgement when interpreting the results.
Warning	Queue variations	Analysis Options	Queue percentiles may be unreliable if the mean queue in any time segment is very low or very high.

Junction Network

Junctions

Junction	Name	Junction type	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS
1	Thirty Road & Young Street (East Junction)	HCM Roundabout		1, 2, 3	6.44	Α
2	Thirty Road & Young Street (West Junction)	HCM Roundabout		1, 2, 3	6.55	А

Junction Network

Driving side	Lighting	Network delay (s)	Network LOS
Right	Normal/unknown	6.49	Α

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D2	2041	PM	PHF	00:00	01:00	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

Demand overview (Traffic)

Junction	Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
	Young Street (W)		✓	460	100.000
Thirty Road & Young Street (East Junction)	Young Street (E)		✓	111	100.000
	Thirty Road (N)		✓	455	100.000
Thirty Road & Young Street (West Junction)	Young Street (E)		✓	430	100.000
	Young Street (W)		✓	227	100.000
	Thirty Road (S)		✓	429	100.000

Peak Hour Factor Data (Traffic)

Junction	Arm	Hourly volume (PCU/hr)	Peak hour factor	Peak time segment	
Thirty Road & Young Street (East Junction)	Young Street (W)	460	0.92	SecondQuarter	
	Young Street (E)	111	0.92	SecondQuarter	
	Thirty Road (N)	455	0.92	SecondQuarter	
Thirty Road & Young Street (West Junction)	Young Street (E)	430	0.92	SecondQuarter	
	Young Street (W)	227	0.92	SecondQuarter	
	Thirty Road (S)	429	0.92	SecondQuarter	

Origin-Destination Data

_		_
Demand ((PCU	/hr)

Demand (1 00m)					
		То			

Thirty Road & Young Street (East Junction)

		Young Street (W)	Young Street (E)	Thirty Road (N)
From	Young Street (W)	0	88	372
From	Young Street (E)	42	0	69
	Thirty Road (N)	388	67	0

Demand (PCU/hr)

Thirty Road & Young Street (West Junction)

	То					
		Young Street (E)	Young Street (W)	Thirty Road (S)		
From	Young Street (E)	0	73	357		
	Young Street (W)	93	0	134		
	Thirty Road (S)	367	62	0		

Vehicle Mix

Thirty Road & Young Street (East Junction)

Heavy Vehicle Percentages

	То					
		Young Street (W)	Young Street (E)	Thirty Road (N)		
From	Young Street (W)	0	4	3		
FIOIII	Young Street (E)	4	0	8		
	Thirty Road (N)	4	3	0		

Heavy Vehicle Percentages

Thirty Road & Young Street (West Junction)

	То					
		Young Street (E)	Young Street (W)	Thirty Road (S)		
From	Young Street (E)	0	5	4		
	Young Street (W)	0	0	1		
	Thirty Road (S)	4	5	0		

Detailed Demand Data

Demand for each time segment

Junction	Arm	Time Segment	Demand (PCU/hr)	Demand in PCU (PCU/hr)	Pedestrian Demand (Ped/hr)
	—	00:00-00:15	433	433	0.00
		00:15-00:30	500	500	0.00
	roung Street (w)	00:30-00:45	473	473	0.00
		00:45-01:00	433	433	(Ped/hr) 0.00
		00:00-00:15	105	105	0.00
Thirty Road & Young Street (East Junction)	Young Street (E)	00:15-00:30	121	121	(PCU/hr) (Ped/hr) 433 0.00 500 0.00 473 0.00 433 0.00 105 0.00 121 0.00 114 0.00 429 0.00 495 0.00 468 0.00 429 0.00 405 0.00 447 0.00 214 0.00 234 0.00 214 0.00 214 0.00
Thirty Road & Foung Street (East Junction)	roung Street (E)	00:30-00:45	114	114	0.00
		00:45-01:00	105	105	(Ped/hr) 0.00
		00:00-00:15	429	429	0.00
	Thirty Road (N)	00:15-00:30	495	495	0.00
		00:30-00:45	468	468	0.00
		00:45-01:00	429	429	0.00
	V	00:00-00:15	405	405	0.00
		00:15-00:30	467	467	0.00
	Young Street (E)	00:30-00:45	442	442	0.00
		00:45-01:00	405	405	0.00
		00:00-00:15	214	214	0.00
Thirty Road & Young Street (West Junction)	Young Street (W)	00:15-00:30	247	247	0.00
	Tourig Street (W)	00:30-00:45	234	234	0.00
		00:45-01:00	214	214	0.00
		00:00-00:15	404	404	0.00
	Thirty Road (S)	00:15-00:30	466	466	0.00

00:30-00:45	441	441	0.00	
00:45-01:00	404	404	0.00	

Results

Results Summary for whole modelled period

Junction	Arm	Max RFC	Max Delay (s)	Max 95th percentile Queue (PCU)	Max LOS
Thirty Road & Young Street (East Junction)	Young Street (W)	0.39	6.69	1.9	А
	Young Street (E)	0.13	5.49	0.5	А
	Thirty Road (N)	0.38	6.41	1.8	А
Thirty Road & Young Street (West Junction)	Young Street (E)	0.36	6.37	1.7	А
	Young Street (W)	0.27	6.63	1.1	А
	Thirty Road (S)	0.37	6.68	1.8	А

Graphs

- Thirty Road & Young Street (East Junction) Young Street (W) Arm Results () Queue95 (PCU)
 Thirty Road & Young Street (East Junction) Young Street (E) Arm Results () Queue95 (PCU)
 Thirty Road & Young Street (East Junction) Thirty Road (N) Arm Results () Queue95 (PCU)
 Thirty Road & Young Street (West Junction) Young Street (E) Arm Results () Queue95 (PCU)
- Thirty Road & Young Street (West Junction) Young Street (W) Arm Results () Queue95 (PCU)
 Thirty Road & Young Street (West Junction) Thirty Road (S) Arm Results () Queue95 (PCU)

